## FLOW MONITORING LITTLE ROCK WASTEWATER

RJN Group, Inc. was retained by Little Rock Wastewater to update a System Evaluation and Capacity Assurance Plan (SECAP) for the wastewater system. As part of this update RJN Group, Inc. conducted a City wide (109 day) flow monitoring study. This Technical Memorandum outlines the approach and the results of the rainfall and flow monitoring.

The 2009-2010 flow monitoring evaluation consisted of:

- Flow/Rainfall Monitoring: sixty-nine (69) flow meters and twenty (20) rain gauges
- Analysis/Report

#### **DESCRIPTION OF EXISTING CONDITIONS**

The existing wastewater collection system for the Study Area contains approximately 6,558,054 linear feet of sanitary sewer line and 35,072 manholes which are owned and maintained by Little Rock Wastewater.

#### **DEFINITIONS AND ABBREVIATIONS**

This section contains definitions and abbreviations commonly used throughout this report.

- (1) <u>Infiltration</u> (as defined by USEPA) the water entering a sewer system and service connections from the ground through such means as, but not limited to, defective pipes, pipe joints, service connections, service laterals, or manhole walls.
- (2) <u>Inflow</u> (as defined by USEPA) the water discharged into a sewer system, including service connections, from such sources as roof leaders; cellar, yard, and area drains; foundation drains; cooling water discharges; drains from springs and swampy areas; manhole covers; cross connections from storm sewers, combined sewers, or catch basins; storm waters; surface runoff; or drainage.
- (3) <u>Excessive infiltration and inflow (I/I)</u> the extraneous clean water that enters the sanitary sewer system which can be eliminated on a cost effective basis.
- (4) <u>Base Flow</u> wastewater flow exclusive of infiltration or inflow. Generally determined from water records during months when most of the water consumption is returned to the wastewater collection system.

- (5) <u>Permanent Infiltration</u> extraneous flow that enters the sewer system through the ground during periods of dry-weather/low-groundwater. Generally determined by subtracting base flow during winter months from the average daily dry-weather monitored flow.
- (6) <u>Peak Infiltration</u> the maximum extraneous flow that enters the wastewater collection system during high groundwater conditions after the inflow effects of a rain event have ended. Generally determined by subtracting dry-weather/low-groundwater flow (average daily dry weather monitored flow) from flow recorded during periods of high groundwater.
- (7) <u>Average Daily Dry-Weather Flow</u> dry-weather/low-groundwater flow exclusive of dry-weather/high-groundwater (peak infiltration) and wet weather (inflow) flow. Includes base flow and permanent infiltration only.
- (8) <u>Average Daily Dry-Weather Flow Peaking Factor</u> the ratio between the peak hourly flow rate and the average daily flow.
- (9) <u>1-Year/60-Minute Storm</u> a storm event that produces 1.55 inches of rain per hour and is expected to occur once in any given year.
- (10) <u>2-Year/60-Minute Storm</u> a storm event that produces 1.80 inches of rain per hour and has a 50 percent probability of occurring in a given year.
- (11) <u>5-Year/60-Minute Storm</u> a storm event that produces 2.32 inches of rain per hour and has a 20 percent probability of occurring in a given year.
- (12) <u>Design Storm Event</u> a storm event selected for purposes of analyzing its effect on the wastewater collection system.
- (13) gpd gallons per day.
- (14) mgd million gallons per day.
- (15) <u>idm</u> inch-diameter-miles. The product of sewer pipe diameter in inches and length of sewer in feet divided by 5,280 feet.
- (16) gpd/idm gallons per day per inch-diameter-mile.
- (17) <u>Surcharge Condition</u> When the sewer flow depth equals or exceeds the diameter of the discharging sewer lines. (WEF Manual of Practice FD-6)
- (18) <u>Infiltration and Inflow (I/I)</u> A combination of infiltration and inflow wastewater volume in sanitary sewer.

#### FLOW MONITORING

Flow monitoring is one of the most important steps in evaluating a sanitary sewer collection system. It is typically performed during Spring and Fall seasons when the chances of rainfall are greater than any other season. The flow monitoring data is used to examine the existing dry and wet weather flows, the affects of rainfall on the wastewater collection system, and the extent of I/I entering the system.

Temporary flow monitoring was performed for a period of (109) days from October 22, 2009 to February 08, 2010. Flow monitoring locations were chosen based upon a previous city wide flow monitoring study performed in 2000. Flow site locations from the previous 2000 study were considered based on suitable hydraulics and installation conditions, locations found unsuitable for monitoring were adjusted. An additional six (6) flow meters were installed giving a more accurate representation of the sewer system flow for modeling purposes. An Area Map denoting the locations of the flow meters and rain gauges are shown in Exhibit A-1 at the back of this report.

Table A-1 lists the location and pipe diameter of the flow metering locations. Flow meter site investigation and installation reports were performed for each monitoring location and are included in the back of this Appendix.

Flow meters were used to record depth of flow and velocity at five-minute intervals. Engineering review and input of additional calibration data was used to finalize the metered flow data. Manual depth and velocity readings (velocity profiles) were taken on a weekly basis to verify the metered data. Average flow rates for one-hour intervals were determined for each monitoring location. The hourly, average flow rates were used to determine daily dry-weather and wet weather flow rates. Flow data collected during rainfall events was evaluated to determine peak instantaneous inflow rates.

#### **RAINFALL MONITORING**

Twenty (20) rain gauges were utilized to determine the amount of rainfall that occurred during these periods. Twelve (12) of the rain gauges were maintained by the Little Rock Wastewater while the remaining eight (8) were installed by RJN Group, Inc.

Historical rainfall intensities data for various storm recurrence intervals is given in Table A-2. Rainfall summaries and the locations of the eight (8) temporary rain gauges are listed in Table A-3.

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Table A-1

## FLOW METER LOCATIONS LITTLE ROCK SECAP

Meter Basin	Manhole Number	Street Location	Manhole Depth (ft)	Average Pipe Diameter (in)
LR001	0F003	9720 Rodney Parham (Butler Park)	9.8	24
LR002	0F013	9720 North Rodney Parham Road	14.5	15
LR002R	18H003	4300 E. 9th Street	3.0	60
LR003	0F146	9300 Treasure Hill Road	7.7	19
LR003R	19I003	900 Temple Street	3.0	58
LR004	1G076	9010 Markham	8.7	12
LR004R	18J001	2400 Dave Grundfest Road	2.0	58
LR005	1G119	300 John Barrow Road	7.9	37
LR005R	9F018	Waterworks Road	42.0	41
LR006	1F056	807 Shae	8.5	12
LR006R	5M019	4800 Mabelvale Pike Road	48.0	48
LR007	2G044	212 Rodney Parham Road	12.7	15
LR007R	6M001	4800 Mabelvale Pike Road	48.0	48
LR008	3H011	7510 Ouchita Drive	13.3	12
LR008R	4N023	South University	4.0	42
LR009	2J066	7800 25th Street	5.5	15
LR009R	6L011	Mabelvale Pike	4.0	48
LR010	2K143	3427 Wynne	5.1	19
LR010R	31007	1801 Leander Road	15.6	43
LR012	3M004	Fairways Drive	7.0	15
LR014	-2E015	1602 Merrill Drive	11.0	18
LR015	-2E010	1602 Merrill Drive	6.6	15
LR018	20003	Hindman Park Golf Course	7.5	36
LR021	14K005	2684 Vance Street	18.3	21
LR023	12K019	3017 Martel Street	8.7	12
LR025	11L023	Crump Park	10.6	24
LR026	11L049	Chester	7.8	12
LR028	9K034	2438 Roosevelt Avenue	9.3	19
LR029	91064	1920 Appianway Street	7.6	15
LR031	6K022	5021 Asher	10.2	15
LR032	6L008	5219 34th Street	12.4	30
LR034	5G158	Near Intersection Of Fair Park Blvd. and Club	9.1	30
		House Drive.		
LR035	5M008	4705 University	18.1	30
LR036	5M009	4705 S. University	6.7	12
LR037	70012	6011 Scott Hamilton Drive	14.7	18
LR043	2R053	8001 Assembly Court	18.1	24
LR044	2Q004	7669 Mabelvale Pike	8.7	24
LR050	15H110	405 Fletcher Street	20.3	53

## FLOW METER LOCATIONS LITTLE ROCK SECAP

Meter Basin	Manhole Number	Street Location	Manhole Depth (ft)	Average Pipe Diameter (in)
LR055	9F024	2326 Cantrell	16.1	23
LR057	6C047	3500 Rebsamen Park Road	4.1	10
LR058	2C112	38 Tallyho	7.3	18
LR059	2B002	Big Dam Bridge	14.0	25
LR062	14L002	Under Intersection Of 440 And Springer	14.8	12
LR063	13I049	722 Frontage Road (Rockefeller Elem. School)	6.2	18
LR100	7K092	3200 Washington	8.1	35
LR101	-8G007	16105 Chenal	15.4	25
LR102	-1L003	4023 ShacklefoRoad Road	10.0	24
LR103	20008	Hindman Park Golf Course	10.7	42
LR104B	20026	Hindman Park Golf Course	10.5	25
LR105	4N014	Yellow City Gate Behind Old Crane Ford Dealership on University.	10.1	42
LR106	11L092	Little Rock Animal Services off of Arch Street	29.6	43
LR107	8Q015	8001 Jamison Road	15.0	25
LR108	6T059	End Of Sunset Lane (Farm Field To The Right, Behind Barn)	72.0	30
LR109	6T057	9526 Reck Road	15.3	29
LR110	4U014	6201 Mabelvale CTF (Cut Off)	5.3	15
LR111	4V001	11301 Gyer Springs Road	18.1	18
LR112	10G066	1600 Cantrell (Inside Episcopal School Enclosure)	18.3	36
LR113	-7-A005	18 Pinnacle Circle	15.5	18
LR114	-8-A002	7820 Cantrell Road	14.5	21
LR116	-10-B008	3 Buckland Drive	6.0	18
LR117	-4T008	13100 Otter Creek Road (In Front Of Cruizzers Car Wash)	25.8	42
LR118	-1Q006	8804 Mabelvale Pike	11.7	24
LR119	-3R004	Look For A Right Away With Power Lines On The Right Side Of W. Baseline.	15.9	12
LR120	-4T002	10100 Block Of Stagecoach Road	20.9	24
LR122	16K009	2900 Bond (Bond And Roosevelt)	20.6	60
LR123	16K005	2900 Bond Street	29.5	60
LR124	8R051	8223 Jamison Road	20.4	35
LR125	8E013	1710 Lilac Circle	7.9	12
LR126	8E099	1704 Lilac Circle	7.2	18

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#### Table A-2

## RAINFALL INTENSITIES FOR VARIOUS STORM RECURRENCE INTERVALS $^{1/}$

Storm Recurrence Interval (Year)	Total Rainfall 60-Minute Duration Storm (in)
1	1.55
2	1.80
5	2.32

1/ Based on National Weather Service Technical Paper No. 40.

#### **DETERMINATION OF AVERAGE DAILY DRY-WEATHER FLOW**

Flow data collected during dry-weather/low-groundwater periods was analyzed to determine the average daily dry-weather flow for each of the sixty-nine (69) basins. The dry-weather period selected for this analysis was from November 14, 2009 through November 21, 2009. The analysis determined that the average daily dry-weather flow during the monitoring period was approximately 34.5 mgd. From this, 1.8 mgd will convey its flow to the new Little Maumelle Treatment Facility, while 32.7 mgd will flow to the Adams and Fourche Treatment Plants

A summary of average daily dry-weather flow by basin is given in Table A-4 and is shown graphically on page A-15. A basin flow diagram giving average daily dry-weather flow is shown on Exhibit A-2. Hydrographs of the dry-weather flow overlaid with wet-weather periods for each basin are included in the back of this Appendix.

#### **AVERAGE DAILY DRY-WEATHER FLOW PEAKING FACTOR**

Wastewater flow during dry-weather periods will vary during the day in response to water consumption. By examining the diurnal curves for each monitored drainage basin, a peaking factor was determined. The peaking factor is the ratio of the peak hourly flow rate and the average daily flow. The average peaking factor for the Study area was 1.73. Peaking factors varied from a minimum of 1.27 to a maximum of 3.13. These are given for each basin in Table A-5 and shown graphically on page A-18.

Table A-3

## RAINFALL SUMMARY

	R	G 01	RO	G 02	R	G 03	RO	G 04
	McClellan	High School	Rede	dy Ice	Endoscopy	Center of Ark.	College St	ation Liquor
		Springs Road	1640 E.	15th Street		University	_	Asher
		Peak		Peak		Peak		Peak
	Total	<b>60-Minute</b>	Total	<b>60-Minute</b>	Total	<b>60-Minute</b>	Total	<b>60-Minute</b>
	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall
	Rainfall	<b>Intensity</b>	Rainfall	Intensity	Rainfall	Intensity	Rainfall	<b>Intensity</b>
Date	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)
2009								
Oct. 12	<u>1</u> /	<u>1</u> / <u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
13	<u>1</u> /	<u>1</u> /	1/ 1/ 1/	1/ 1/ 1/	1/ 1/ 1/	<u>1</u> / <u>1</u> / <u>1</u> /	<u>1</u> / <u>1</u> / <u>1</u> /	1/ 1/ 1/
14	0.00	0.00	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
15	0.13	0.09	<u>1</u> /	<u>1</u> /	0.13	0.10	0.14	0.13
22	1.77	0.33	1.95	0.32	1.95	0.38	1.74	0.24
26	0.27	0.05	0.25	0.05	0.28	0.06	0.25	0.05
27	0.97	0.30	0.89	0.23	0.98	0.29	0.91	0.29
29	4.11	1.98	4.02	1.36	4.30	2.15	4.14	1.92
30	0.86	0.14	0.88	0.15	0.96	0.19	0.08	0.17
Nov. 16	0.22	0.14	0.23	0.17	0.31	0.21	0.30	0.18
20	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
24	0.03	0.02	0.03	0.02	0.04	0.04	0.06	0.04
25	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
29	0.89	0.64	0.89	0.64	0.63	0.45	0.60	0.43
30	0.01	0.01	0.01	0.01	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
Dec. 02	1.62	0.30	1.62	0.30	1.49	0.27	1.54	0.29
06	0.05	0.02	0.05	0.02	0.06	0.03	0.04	0.02
07	0.01	0.01	0.01	0.01	0.02	0.01	0.01	0.01
08	0.96	0.46	0.96	0.46	1.03	0.55	0.84	0.47
12	0.22	0.11	0.22	0.11	0.17	0.07	0.17	0.07
13	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	0.01	0.01

/ No rainfall recorded.

## **RAINFALL SUMMARY**

	McClellan	G 01 High School Springs Road	Rede	G <b>02</b> dy Ice 15th Street	Endoscopy	G 03 Center of Ark. University	College St	G 04 ation Liquor Asher
Date	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)
Dec. 22	` '	` '	0.07	0.06	0.04	0.02		` ,
23	<u>1</u> /	<u>1</u> /	4.57	0.73	4.34	0.58	<u>1</u> /	<u>1</u> /
23	1/ 1/ 1/ 1/	1/ 1/ 1/ 1/	5.28	0.73	4.34	0.56	1/ 1/ 1/ 1/	1/ 1/ 1/ 1/
25	1/ 1/	<u>1</u> / 1/	1/	0.04 <u>1</u> /	0.01	0.01	<u>1</u> / 1/	<u>1</u> / 1/
30	1/ 1/	<u>1</u> / 1/	0.11	0.10	0.02	0.01	<u>1</u> / 1/	<u>1</u> / 1/
2010	<u>1</u> /	1/	0.11	0.10	0.02	0.01	1/	1/
Jan. 04	<u>1</u> /	<u>1</u> /	0.01	0.01	0.01	0.01	<u>1</u> /	<u>1</u> /
07	<u>1</u> /	<u>1</u> /	0.01	0.01	1/	1/	<u>1</u> /	<u>1</u> /
16	0.25	0.05	0.19	0.05	0.21	0.05	0.21	0.05
17	0.58	0.16	0.73	0.16	0.63	0.15	0.54	0.14
19	1/	1/	1/	1/	1/	1/	1/	<u>1</u> /
20	0.82	0.19	0.74	0.34	0.87	0.16	0.83	0.41
21	0.23	0.19	0.12	0.10	0.06	0.04	0.11	0.08
23	0.70	0.64	0.87	0.83	0.72	0.69	0.75	0.72
24	<u>1</u> /	<u>1</u> /						
30	0.01	0.01	$0.\overline{01}$	0.01	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
31	0.31	0.12	0.61	0.20	0.69	0.25	0.41	0.22
Feb. 01	<u>1</u> /	<u>1</u> /						
04	0.87	0.13	$0.\overline{8}_{3}$	0.11	0.83	0.10	0.68	0.09
05	0.19	0.06	0.23	0.07	0.25	0.07	0.20	0.05
06	<u>1</u> /	<u>1</u> /						

<sup>1/</sup> No rainfall recorded.

## **RAINFALL SUMMARY**

	R	G 01	RO	G 02	R	G 03	RO	G <b>04</b>
	McClellan	High School	Rede	dy Ice	Endoscopy	Center of Ark.	College St	ation Liquor
	9417 Geyer	Springs Road	1640 E.	15th Street	1024 N.	University	5301	Asher
		Peak		Peak		Peak		Peak
	Total	60-Minute	Total	60-Minute	Total	60-Minute	Total	60-Minute
	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall
	Rainfall	<b>Intensity</b>	Rainfall	Intensity	Rainfall	<b>Intensity</b>	Rainfall	Intensity
Date	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)
Feb. 08	0.80	0.30	1.04	0.39	0.64	0.35	0.97	0.35
09	0.68	0.12	0.35	0.05	0.65	0.11	0.57	0.12
10	0.33	0.07	0.27	0.08	0.27	0.15	0.21	0.06
11	0.13	0.03	0.17	0.05	0.13	0.04	0.10	0.03
12	0.05	0.05	0.17	0.17	0.01	0.01	0.04	0.04
14	0.18	0.08	0.22	0.11	0.21	0.12	0.21	0.12
Total	18.25		28.61		27.20		16.66	

<sup>1/</sup> No rainfall recorded.

## RAINFALL SUMMARY

	Ron Pa	<b>G 05</b> ack Carpet 4 I-30	Gan	G <b>06</b> nestop enal Pkwy	Verizon	G 07 Store - STE antrell Road	LR Rac	G 08 quet Club gton Road
Date	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)	Total Daily Rainfall (in)	Peak 60-Minute Rainfall Intensity (in/hr)
2009	(111)	(111/111)	(111)	(111/111)	(111)	(111/111)	(111)	(111/111)
Oct. 12	1/	1/ 1/ 1/	0.03 1.84	0.01 0.56	0.04 1.73	0.02 0.49	0.04 1.71	0.01 0.51
13	<u>1</u> / <u>1</u> /	<u>1</u> / 1/	1.64 <u>1</u> /	0.36 <u>1</u> /	0.02	0.49	0.02	0.51
15	0.18	0.15	0.14	0.11	0.02	0.15	0.02	0.11
22	1.85	0.25	2.31	0.36	2.61	0.36	1.86	0.26
26	0.26	0.05	0.37	0.08	0.44	0.08	0.29	0.07
27	0.91	0.28	1.00	0.27	1.01	0.24	0.90	0.28
29	4.34	1.91	2.49	0.89	1.87	0.54	3.80	1.65
30	0.69	0.16	0.78	0.19	0.87	0.18	0.81	0.18
Nov. 16	0.26	0.13	0.31	0.21	0.25	0.14	0.31	0.22
20	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	0.04	0.02	<u>1</u> /	<u>1</u> /
24	0.05	0.02	0.03	0.03	0.03	0.03	0.03	0.03
25	<u>1</u> /	<u>1</u> /	0.05	0.05	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
29	0.92	0.75	0.64	0.42	1.07	0.69	0.75	0.47
30	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
Dec. 02	1.62	0.31	1.60	0.28	1.48	0.24	1.33	0.21
06	0.06	0.03	0.06	0.03	0.10	0.05	0.07	0.04
07	0.02	0.01	0.01	0.01	0.02	0.01	0.01	0.01
08	0.96	0.53	0.90	0.49	1.26	0.79	0.91	0.51
12	0.20	0.09	0.14	0.06	0.20	0.08	0.15	0.06
13	1/	1/	1/	1/	1/	1/	1/	1/

1/ No rainfall recorded.

## RAINFALL SUMMARY

	Ron Pa	<b>G 05</b> ck Carpet 4 I-30	Gan	G 06 nestop enal Pkwy	Verizon	G 07 Store - STE ntrell Road	LR Rac	G 08 Juet Club gton Road
	Total Daily Rainfall	Peak 60-Minute Rainfall Intensity	Total Daily Rainfall	Peak 60-Minute Rainfall Intensity	Total Daily Rainfall	Peak 60-Minute Rainfall Intensity	Total Daily Rainfall	Peak 60-Minute Rainfall Intensity
Date	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)
Dec. 22	0.07	0.06	0.13	0.02	<u>1</u> /	<u>1</u> /	0.03	0.02
23	4.03	0.52	5.14	0.71	4.97	0.80	4.49	0.70
24	4.17	0.66	4.20	0.52	4.88	0.68	3.64	0.47
25	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
30	0.03	0.01	0.01	0.01	0.01	0.01	0.02	0.01
2010								
Jan. 04	<u>1</u> /	<u>1</u> /	0.01	0.01	0.01	0.01	0.01	0.01
07	0.01	0.01	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
16	0.27	0.06	0.15	0.04	0.22	0.05	0.20	0.05
17	0.54	0.16	0.54	0.14	0.55	0.13	0.55	0.13
19	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /	0.05	0.05
20	0.75	0.17	0.77	0.12	0.80	0.45	0.64	0.12
21	0.18	0.12	0.09	0.06	0.12	0.09	0.06	0.04
23	0.73	0.70	0.72	0.65	0.93	0.77	0.76	0.71
24	0.01	0.01	0.01	0.01	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
30	0.03	0.02			0.040	0.02	0.03	0.02
31	0.38	0.18	0.53	0.20	0.72	0.26	0.50	0.21
Feb. 01	<u>1</u> /	<u>1</u> /	0.01	0.01	<u>1</u> /	<u>1</u> /	<u>1</u> /	<u>1</u> /
04	$0.\overline{8}2$	0.13	0.79	0.11	0.96	0.13	0.66	0.09
05	0.19	0.05	0.22	0.06	0.23	0.07	0.22	0.05
06	<u>1</u> /	<u>1</u> /	1/	<u>1</u> /	0.01	0.01	<u>1</u> /	<u>1</u> /

<u>1</u>/ No rainfall recorded.

## RAINFALL SUMMARY

	R	G 05	Re	G 06	R	G 07	RO	G 08
	Ron Pa	ck Carpet	Gan	nestop	Verizon	Store - STE	LR Rac	quet Club
	9524	4 I-30	1329 Che	enal Pkwy	4524 Ca	ntrell Road	1 Huntin	gton Road
		Peak		Peak		Peak		Peak
	Total	<b>60-Minute</b>	Total	<b>60-Minute</b>	Total	<b>60-Minute</b>	Total	60-Minute
	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall	Daily	Rainfall
	Rainfall	<b>Intensity</b>	Rainfall	<b>Intensity</b>	Rainfall	Intensity	Rainfall	Intensity
Date	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)	(in)	(in/hr)
Feb. 08	0.89	0.31	0.04	0.01	0.86	0.38	0.37	0.29
09	0.40	0.06	0.52	0.14	0.27	0.04	0.39	0.13
10	0.16	0.07	0.30	0.11	0.12	0.06	0.33	0.17
11	0.10	0.03	0.11	0.04	0.16	0.05	<u>1</u> /	<u>1</u> /
12	0.31	0.13	0.12	0.08	0.43	0.17	<u>1</u> /	<u>1</u> /
14	0.18	0.08	0.20	0.12	0.18	0.10	0.19	0.11
Total	26.58		27.32		29.70		26.27	

1/ No rainfall recorded.

#### Table A-4

## AVERAGE DAILY DRY-WEATHER FLOW LITTLE ROCK SECAP DRY WEEK 11/14/2009 TO 11/21/2009

	Cumulative Average Daily Dry-Weather Flow	Basin Average Daily Dry-Weather Flow
Meter Basin	(mgd)	(mgd)
Little Maumelle Service Area	0.510	0.510
L113	0.518	0.518
L114	0.989	0.989
L116	0.311	0.311
Subtotal		1.818
Adams & Fourche Service Areas		
L001 & L002	1.511	0.704
L001	1.393	1/
L002	0.118	<u>1</u> /
L002R	8.032	$0.\overline{071}$
L003	0.541	0.541
L003R	12.983	0.276
L004	0.188	0.188
L004R	1.557	0.127
L005	2.579	2.146
L005R	3.503	0.884
L006	0.089	0.089
L007	0.326	0.326
L008	0.062	0.062
L008R	3.685	0.555
L009	0.312	0.312
L009R	10.221	0.608
L010	0.462	0.462
L010R	6.881	3.096
L012	0.179	0.179
L014	0.549	0.549
L015	0.258	0.258
L018	1.422	0.302
L021	0.626	0.297
L023	0.133	0.133
L025	0.365	0.365
L026	0.055	0.055
L028	0.295	0.111
L029	0.184	0.184
L031	0.102	0.102
L032	2.511	1.235
L034	1.276	1.276

1/ Included in combined meters.

## AVERAGE DAILY DRY-WEATHER FLOW LITTLE ROCK SECAP DRY WEEK 11/14/2009 TO 11/21/2009

	Cumulative Average Daily Dry-Weather Flow	Basin Average Daily Dry-Weather Flow
Meter Basin	(mgd)	(mgd)
L035	0.267	0.267
L036	0.161	0.161
L037	0.336	0.336
L043	0.455	0.455
L044	0.974	0.519
L050	7.961	0.736
L055	1.311	0.611
L057	0.095	0.095
L058	0.773	0.773
L059	0.706	0.067
L062	0.125	0.125
L063	0.329	0.329
L100	0.373	0.373
L101	0.433	0.433
L102	1.120	1.120
L103	1.791	0.576
L104B & L105	2.144	1.842
L104B	0.365	<u>1</u> /
L105	1.779	<u>1</u> /
L107	0.622	0.622
L108	0.384	0.384
L109	0.222	0.222
L110	0.259	0.259
L111	0.115	0.115
L112	2.411	2.411
L117	0.432	0.432
L118	0.244	0.244
L119	0.218	0.218
L120	0.321	0.321
L122 & 123	14.137	2.317
L122	1.430	<u>1</u> / <u>1</u> /
L123	12.707	<u>1</u> /
L124	1.132	0.152
L125	0.312	0.312
L126	0.388	0.388
Subtotal		<u>32.708</u>
Total		34.526

1/ Included in combined meter basins.

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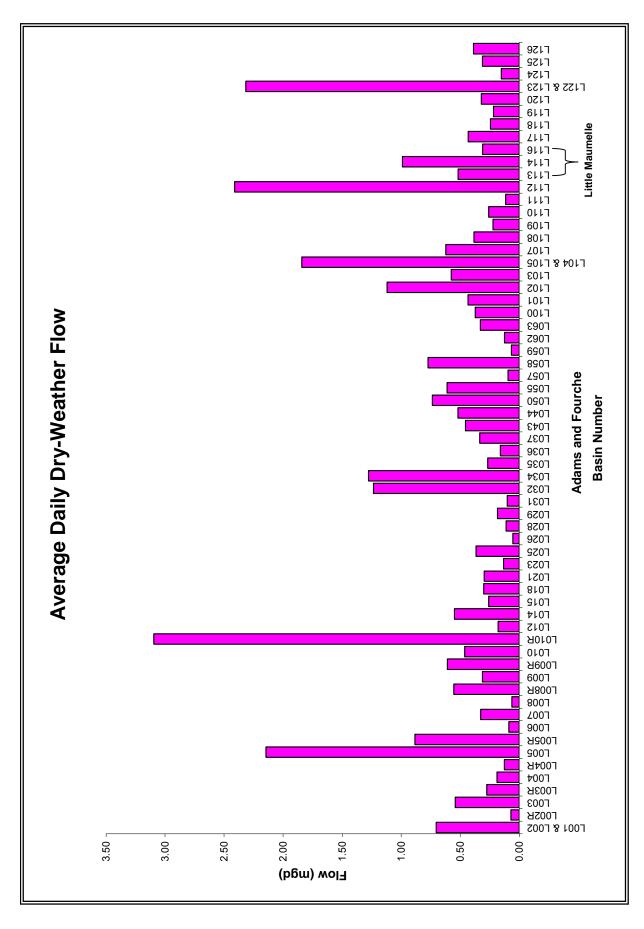


Table A-5

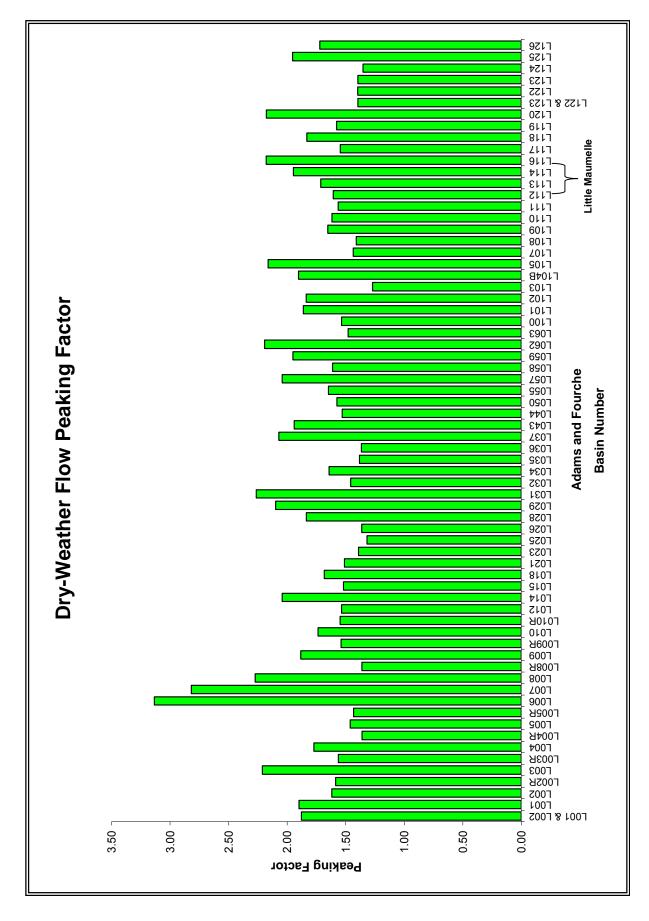
## DRY-WEATHER FLOW PEAKING FACTOR LITTLE ROCK SECAP DRY WEEK 11/14/2009 TO 11/21/2009

Motor Posin	Cumulative Average Daily Flow Rate	Cumulative Peak Hourly Flow Rate	Cumulative Average Daily
Meter Basin	(mgd)	(mgd)	Peaking Factor
L113	0.518	0.888	1.71
L113 L114	0.989	1.925	1.71
L114 L116	0.311	0.678	
Subtotal	0.311	0.078	2.18 1.95
Subiblai			(average)
Adams & Fourche Serv	rice Areas		(average)
L001 & L002	1.511	2.838	1.88
L001	1.393	2.647	1.90
L002	0.118	0.191	1.62
L002R	8.032	12.739	1.59
L003	0.541	1.197	2.21
L003R	12.983	20.306	1.56
L004	0.188	0.333	1.77
L004R	1.557	2.120	1.36
L005	2.579	3.770	1.46
L005R	3.503	5.020	1.43
L006	0.089	0.279	3.13
L007	0.326	0.919	2.82
L008	0.062	0.141	2.27
L008R	3.685	5.020	1.36
L009	0.312	0.588	1.88
L009R	10.221	15.740	1.54
L010	0.462	0.802	1.74
L010R	6.881	10.652	1.55
L012	0.179	0.275	1.54
L014	0.549	1.122	2.04
L015	0.258	0.392	1.52
L018	1.422	2.395	1.68
L021	0.626	0.946	1.51
L023	0.133	0.185	1.39
L025	0.365	0.481	1.32
L026	0.055	0.075	1.36
L028	0.295	0.542	1.84
L029	0.184	0.386	2.10
L031	0.102	0.231	2.26
L032	2.511	3.660	1.46
L034	1.276	2.096	1.64

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## DRY-WEATHER FLOW PEAKING FACTOR LITTLE ROCK SECAP DRY WEEK 11/14/2009 TO 11/21/2009

	Cumulative Average Daily Flow Rate	Cumulative Peak Hourly Flow Rate	Cumulative Average Daily
<b>Meter Basin</b>	(mgd)	(mgd)	Peaking Factor
L035	0.267	0.369	1.38
L036	0.161	0.220	1.37
L037	0.336	0.696	2.07
L043	0.455	0.883	1.94
L044	0.974	1.490	1.53
L050	7.961	12.540	1.58
L055	1.311	2.160	1.65
L057	0.095	0.194	2.04
L058	0.773	1.246	1.61
L059	0.706	1.377	1.95
L062	0.125	0.274	2.19
L063	0.329	0.487	1.48
L100	0.373	0.573	1.54
L101	0.433	0.806	1.86
L102	1.120	2.059	1.84
L103	1.791	2.275	1.27
L104B	0.365	0.695	1.90
L105	1.779	3.850	2.16
L107	0.622	0.893	1.44
L108	0.384	0.541	1.41
L109	0.222	0.367	1.65
L110	0.259	0.419	1.62
L111	0.115	0.180	1.57
L112	2.411	3.873	1.61
L117	0.432	0.668	1.55
L118	0.244	0.447	1.83
L119	0.218	0.344	1.58
L120	0.321	0.699	2.18
L122 & L123	14.137	19.746	1.40
L122	1.430	1.999	1.40
L123	12.707	17.747	1.40
L124	1.132	1.532	1.35
L125	0.312	0.610	1.96
L126	0.388	0.668	<u>1.72</u>
Subtotal			<u>1.72</u>
Total			1.73
			Average



#### **INFILTRATION CONDITIONS**

Infiltration may enter the system through pipe joints, sewer line defects (including main sewer lines and building sewer lines), and defective manhole walls, benches, and pipe seals. There are two types of infiltration that can be determined during a study, permanent infiltration and peak infiltration. Permanent infiltration is defined as extraneous flow that enters the sewer system through the ground during periods of dry-weather and low-groundwater. Peak infiltration is defined as the maximum extraneous flow that enters the sanitary sewer system during high-groundwater conditions after the inflow effects of a rain event have ended. Peak infiltration was used to evaluate the effects of infiltration on the sewer system.

#### **DETERMINATION OF PEAK INFILTRATION**

Determining peak infiltration requires analysis of flow data obtained during dry-weather/high-groundwater conditions. Care must be exercised in the analysis to exclude days that are too close to rainfall events. This is necessary to avoid including residual inflow (rainfall induced infiltration) that may lead to an over-estimation of peak infiltration. Generally, periods following significant rainfall, excluding the day immediately following a rain event, are used for determining peak infiltration.

Due to high amounts of rainfall that occurred during the study, it was determined that groundwater conditions were favorable during the monitoring period. Analyses of the flow data following major rain events indicated significant infiltration entering the wastewater collection system. A number of basins for example, L122, L123, and L124 exhibit prolong surcharging that occurred for 3-4 days. In these cases an alternative storm event was analyzed when available for infiltration analysis.

For this study, each basin is compared relative to the other by expressing the infiltration rate in units of gpd/idm. It was determined that eighteen (18) basins exhibited significant infiltration rates over 5,000 gpd/idm. The system resulted in a total peak infiltration rate of 36.7 mgd. This is comprised of 2.0 mgd from the Little Maumelle Sewershed and 34.7 mgd from the Adams and Fourche tributary areas. Table A-6 provides the results of the analysis. A basin flow diagram showing peak infiltration is shown on Exhibit A-3. Also, a ranking is given in Table A-6 where one (1) is the most severe.

#### **INFLOW CONDITIONS**

Inflow in a sanitary sewer system is defined as extraneous flow that is a direct result of stormwater runoff. Inflow may enter the sanitary sewer system through directly connected downspouts, area drains, cleanouts, and building sewers. Stormwater may also enter the system through direct or indirect connections between the sanitary sewers and storm drains or ditches and sewer line defects, and through defective manhole covers, frame seals, corbels, and manhole walls.

#### Table A-6

#### SUMMARY OF PEAK INFILTRATION

Meter Basin	Basin Inch-Diameter-Mile (idm)	Cumulative Peak Monitored infiltration (mgd)	Basin Peak Monitored infiltration (mgd)	Basin Peak Unit Infiltration (gpd/idm)	Basin Peak Unit Infiltration Ranking
Little Maumelle		( <b>8</b> )	( <b>g</b> )	( <b>91</b> )	<b>_</b>
L113	213.97	0.516	0.516	2,412	36
L114	581.32	1.088	1.088	1,871	39
L116 <sup>1</sup> /	188.80	0.383	0.383	2,029	38
Subtotal			1.987	,	
Adams & Fourd	che Service Areas				
L001 & L002	188.66	0.989	0.401	2,124	37
L001	<u>2</u> / <u>2</u> /	0.939	<u>2</u> /	<u>2</u> /	<u>2</u> /
L002	<u>2</u> /	0.050	<u>2</u> /	<u>2</u> /	<u>2</u> / <u>2</u> / 4
L002R	116.23	4.201	1.169	10,058	4
L003	174.16	0.646	0.646	3,710	25
L003R	383.03	11.221	3.649	9,527	5
L004	54.59	0.329	0.329	6,020	13
$L004R^{3/4/}$	147.76	15.861	<u>3</u> /	<u>3</u> /	<u>3</u> /
L005	700.72	1.050	0.870	1,242	45
L005R	537.13	1.974	<u>4</u> /	<u>4</u> /	<u>9</u> /
L006	53.28	0.190	0.190	3,562	27
L007	111.51	0.764	0.764	6,852	11
L008	170.12	0.133	0.133	780	50
L008R <sup>5/</sup>	218.00	2.529	0.316	1,449	43
L009	96.03	0.239	0.239	2,488	35
L009R <sup>6</sup> /	354.73	3.775	<u>6</u> /	<u>6</u> /	<u>5</u> /
L010	181.85	0.565	0.565	3,105	30
L010R	374.47	6.022	1.922	5,133	17
L012	62.73	0.275	0.275	4,377	20
L014	126.22	0.490	0.490	3,880	24
L015	62.54	0.098	0.098	1,573	41
L018	227.91	1.335	0.635	2,788	32

Used alternate Infiltration period.

Included in combined meters.

Bypass connection upstream precludes net Infiltration calculation. Value most likely underestimates infiltration.

Storage at Little Maumelle Lift Station precludes net infiltration calculation.

Partial infiltration diverts South to Arch.

Sewer line upstream was surcharged during Infiltration period.

Additional flow diverted from L034 to a Lift Station most likely results in over estimate of the net Infiltration.

Overflow upstream precludes net Infiltration calculation

<sup>1/</sup> 2/ 3/ 4/ 5/ 6/ 7/ 8/ 9/ Some flow from L009R is being diverted to a lift station; therefore, meters L122 and L123 infiltration may be an overestimate.

#### SUMMARY OF PEAK INFILTRATION

Meter Basin	Basin Inch-Diameter-Mile (idm)	Cumulative Peak Monitored infiltration (mgd)	Basin Peak Monitored infiltration (mgd)	Basin Peak Unit Infiltration (gpd/idm)	Basin Peak Unit Infiltration Ranking
L021 <sup>5/</sup>	87.68	0.298	5/	5/	4/
L023	32.84	0.192	$0.\overline{192}$	5,842	<del>-</del> 14
L025	160.49	1.291	1.291	8,044	8
L026	26.35	0.196	0.196	7,422	9
L028	51.07	0.435	< 0.001	<10	56
L029	82.29	0.435	0.435	5,282	16
L031	172.53	0.093	0.093	538	53
L032 <sup>7/</sup>	395.94	2.113	1.031	2,604	33
L034	251.37	1.082	1.082	4,305	21
L035	67.75	0.387	0.387	5,711	15
L036	29.13	0.297	0.297	10,187	3
L037	104.05	0.156	0.156	1,501	42
L043	169.85	0.569	0.569	3,350	28
L044	140.29	0.677	0.108	770	51
$L050^{5/}$	406.48	3.032	<u>5</u> /	<u>5</u> /	<u>5</u> /
L055	117.64	1.072	< 0.001	<10	57
L057	50.36	0.049	0.049	973	47
L058	139.32	0.459	0.459	3,297	29
L059	125.00	1.030	0.571	4,566	19
L062	40.35	0.464	0.464	11,489	1
L063	77.69	0.394	0.394	5,066	18
L100	133.88	0.573	0.573	4,276	22
L101	360.83	0.180	0.180	497	54
L102	552.05	0.700	0.700	1,268	44
$L103^{8/}$	422.88	0.940	<u>8</u> /	<u>8</u> /	<u>7</u> /
L104B & L105	59.28	1.282	0.646	10,905	<u>7/</u> 2
L104B	<u>2</u> /	0.596	<u>2</u> /	<u>2</u> /	2/ 2/ 23
L105	<u>2</u> /	0.686	<u>2</u> /	<u>2</u> /	<u>2</u> /
L107	95.83	0.376	0.376	3,920	23
L108	286.35	0.167	0.167	583	52

*Used alternate Infiltration period.* 

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Included in combined meters.

<sup>1/</sup> 2/ 3/ 4/ 5/ 6/ 7/ 8/ 9/ Bypass connection upstream precludes net Infiltration calculation. Value most likely underestimates infiltration.

Storage at Little Maumelle Lift Station precludes net infiltration calculation.

Partial infiltration diverts South to Arch.

Sewer line upstream was surcharged during Infiltration period.

Additional flow diverted from L034 to a Lift Station most likely results in over estimate of the net Infiltration.

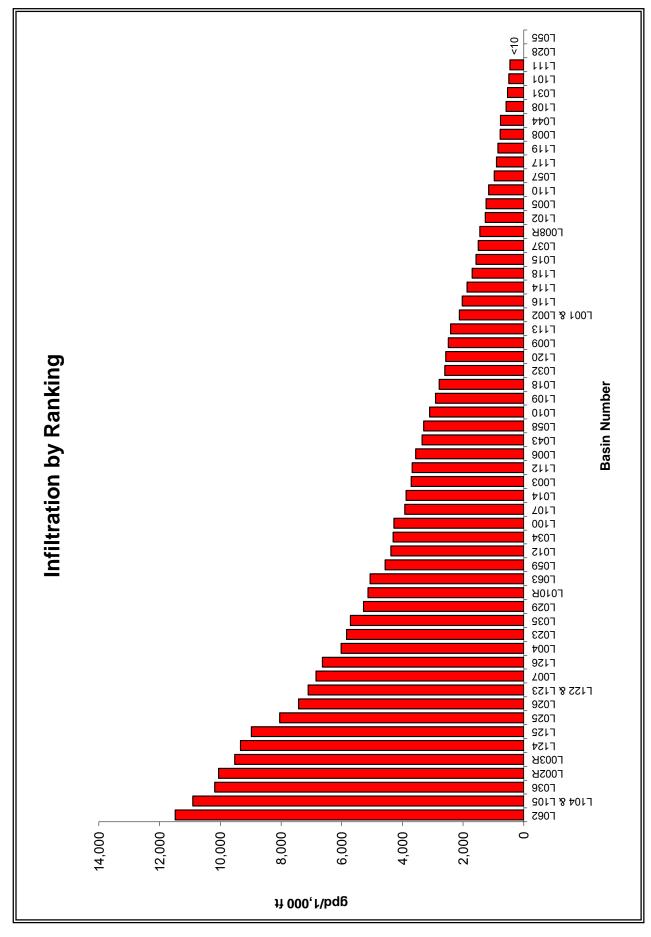
Overflow upstream precludes net Infiltration calculation

Some flow from L009R is being diverted to a lift station; therefore, meters L122 and L123 infiltration may be an overestimate.

#### SUMMARY OF PEAK INFILTRATION

	Basin Inch-Diameter-Mile	Cumulative Peak Monitored infiltration	Basin Peak Monitored infiltration	Basin Peak Unit Infiltration	Basin Peak Unit Infiltration
Meter Basin	(idm)	(mgd)	(mgd)	(gpd/idm)	Ranking
L109	51.07	0.148	0.148	2,906	31
L110	149.21	0.173	0.173	1,161	46
L111	170.27	0.078	0.078	459	55
L112	479.59	1.763	1.763	3,676	26
L117	388.81	0.351	0.351	903	48
L118	139.00	0.236	0.236	1,700	40
L119	89.84	0.077	0.077	852	49
L120	145.94	0.375	0.375	2,570	34
L122 & L123 <sup>9/</sup>	847.49	11.915	6.023	7,107	10
L122	<u>2</u> /	4.343	<u>2</u> /	<u>2</u> /	<u>2</u> /
L123	<u>2</u> /	7.572	<u>2</u> /	<u>2</u> /	<u>2</u> / <u>2</u> /
L124 <sup>1/</sup>	139.69	1.871	1.304	9,336	6
L125	75.27	0.676	0.676	8,981	7
L126	60.99	0.405	0.405	6,641	12
Subtotal			<u>34.713</u>		
Total			36.700		

- Used alternate Infiltration period.
- Included in combined meters.
- Bypass connection upstream precludes net Infiltration calculation. Value most likely underestimates infiltration.
- Storage at Little Maumelle Lift Station precludes net infiltration calculation.
- Partial infiltration diverts South to Arch.
- Sewer line upstream was surcharged during Infiltration period.
- Additional flow diverted from L034 to a Lift Station most likely results in over estimate of the net Infiltration.
- Overflow upstream precludes net Infiltration calculation
- 1/ 2 3/ 4/ 5/ 6/ 7/ 8/ 9/ Some flow from L009R is being diverted to a lift station; therefore, meters L122 and L123 infiltration may be an overestimate.



#### **DETERMINATION OF INFLOW**

Flow data collected during wet-weather periods was analyzed to determine peak inflow originating in each basin. To determine the peak inflow rate, the calibrated model was used to perform the analysis. By isolating each basin's inflow component and treating its outfall as free flow, the peak design inflow rates were calculated. Several storm events of various intensities were used to calibrate the model. This ensures that the model has appropriately distributed inflow.

The analysis projected the peak 1-year storm inflow (1.55 inches/hour) to be 263.358 mgd; 11.4 mgd is generated from the Little Maumelle Sewershed while 252.0 mgd is from the Adams and Fourche Sewersheds. The system overall exhibited severe inflow with rates exceeding 10,000 gpd/linear foot of 1-year/60-minute inflow. A summary of the projected peak wet-weather flow rates during a 1-year/60-minute storm event is given in Table A-7 and is shown graphically on page A-27. The basin unit inflow rate expresses the magnitude of peak inflow relative to other basins. A basin flow diagram giving 1-year/60-minute storm inflow rates are shown on Exhibit A-4.

During the flow monitoring period, seven (7) rain events were recorded with peak 60-minute rainfall intensity greater than 0.25 inches/hour. These intensities ranged from 0.25 inches/hour to 2.00 inches/hour.

#### **TOTAL PEAK 1-YEAR STORM FLOW**

The total peak wet-weather flow projected to occur during a 1-year storm event for Little Rock is approximately 353.721 mgd. This consists of 53.663 mgd of peak hourly dry-weather flow, 36.700 mgd of peak infiltration, and 263.358 mgd of 1-year inflow. Based on an average daily dry-weather flow of 34.526 mgd, this would result in a wet-weather peaking factor average of approximately 10.2. A summary of the projected peak wet-weather flow rates during a 1-year/60-minute storm event is given in Table A-8. Peaking factors varied from 3.0 to 59.3 and are given for each basin in Table A-8 and shown graphically on page A-31.

Table A-7

## SUMMARY OF INFLOW RATES<sup>1</sup>

	Length	Inflow	Basin Peak 1-Year/60-Minute Inflow	Inflow per 1000 lf					
Meter Basin	(lf)	(mgd)	(mgd)	(gpd/1,000 ft)	Rank				
	<u>Little Maumelle Service Area</u>								
L113	130,444	3.088	3.088	23,673	50				
L114	355,738	6.846	6.846	19,246	52				
L116	115,433	1.444	1.444	<u>12,505</u>	56				
Subtotal	601,615		11.378	18,475					
Adams & Fourch	he Service Are	eas							
L001 & L002	107,760	12.006	<u>2</u> /	60,722	18				
L001	93,497	9.642	<u>2</u> /	56,021	22				
L002	14,263	2.364	1.306	91,543	4				
L002R	30,510	72.776	0.774	25,384	47				
L003	116,219	6.656	6.656	57,272	20				
L003R	111,543	95.415	0.791	7,094	65				
L004	38,772	2.188	2.188	56,428	21				
L004R	29,154	57.727	0.905	31,043	38				
L005	384,290	11.462	11.462	29,826	40				
L005R	250,535	22.603	2.108	8,414	62				
L006	38,318	1.596	1.596	41,652	30				
L007	80,149	6.043	6.043	75,398	12				
L008	115,577	0.907	0.907	7,850	63				
L008R	109,549	19.646	<u>2</u> /	24,805	48				
L008R & L105	127,548	31.556	3.429	64,347	16				
L009	61,453	2.215	2.215	36,049	35				
L009R	119,045	82.292	2.862	24,044	49				
L010	125,763	7.786	7.786	61,910	17				
L010R	153,201	40.858	16.467	107,488	3				
L012	45,102	2.911	2.911	64,552	15				
L014	82,343	4.404	4.404	53,486	24				
L015	38,344	1.058	1.058	27,593	44				
L018	105,405	8.481	1.942	18,424	53				
L021	54,381	4.665	0.515	9,477	61				
L023	23,446	2.036	2.036	86,830	6				
L025	111,101	6.055	6.055	54,496	23				
L026	18,654	0.528	0.528	28,307	43				
L028	33,168	6.820	2.739	82,583	7				
L029	58,261	4.081	4.081	70,050	14				
L031	107,594	2.837	2.837	26,363	45				

Based on Model inflow analysis.

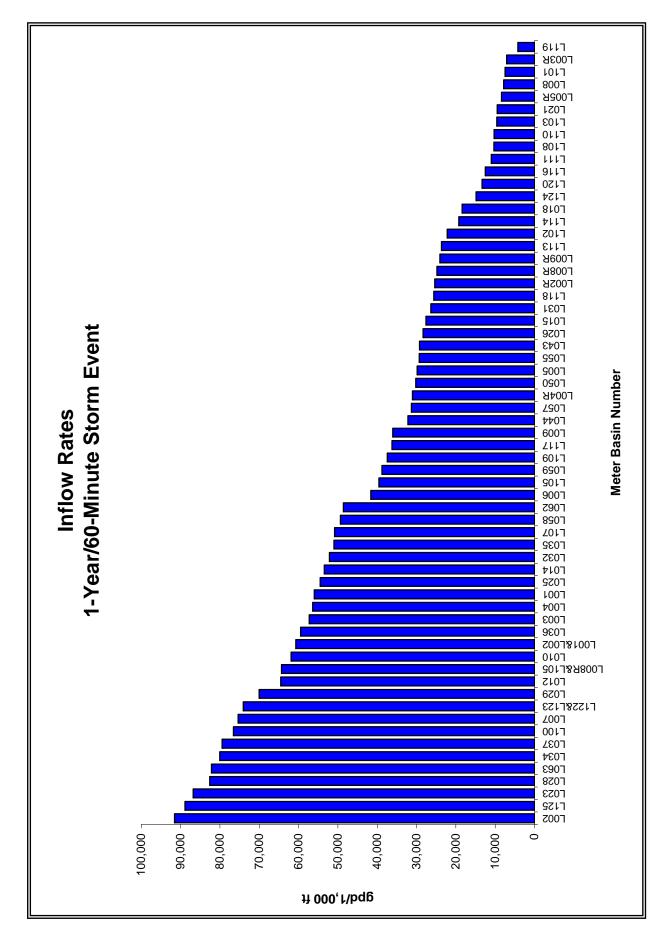
<sup>&</sup>lt;u>1/</u> <u>2/</u> Included in the combined basin.

## SUMMARY OF INFLOW RATES<sup>1</sup>/

	Length	Cumulative Peak 1-Year/60-Minute Inflow	Basin Peak 1-Year/60-Minute Inflow	Basin Peak 1-Year/60-Minute Inflow per 1000 lf	
Meter Basin	(lf)	(mgd)	(mgd)	(gpd/1,000 ft)	Rank
L032	195,215	23.277	10.180	52,149	25
L034	163,627	13.097	13.097	80,040	9
L035	38,056	1.941	1.941	51,009	26
L036	20,145	1.199	1.199	59,501	19
L037	63,633	5.055	5.055	79,447	10
L043	105,458	3.082	3.082	29,227	42
L044	82,650	5.741	2.659	32,174	36
L050	172,196	72.001	5.202	30,209	39
L055	65,814	11.963	1.931	29,342	41
L057	35,302	1.106	1.106	31,328	37
L058	96,033	4.742	4.742	49,375	28
L059	84,210	8.011	3.269	38,824	32
L062	27,497	1.336	1.336	48,600	29
L063	50,512	4.150	4.150	82,155	8
L100	89,605	6.860	6.860	76,558	11
L101	213,600	1.598	1.598	7,482	64
L102	294,477	6.539	6.539	22,205	51
L103	142,557	11.187	1.368	9,593	60
L105	17,999	9.193	<u>2</u> /	39,542	31
L107	52,335	2.661	2.661	50,851	27
L108	158,630	1.639	1.639	10,330	58
L109	34,854	1.305	1.305	37,452	33
L110	88,768	0.911	0.911	10,259	59
L111	95,830	1.052	1.052	10,976	57
L112	291,009	32.234	32.234	110,765	2
L117	179,336	6.503	6.503	36,263	34
L118	72,639	1.860	1.860	25,604	46
L119	55,546	0.235	0.235	4,223	66
L120	91,447	1.222	1.222	13,365	55
L122 & L123	199,000	136.706	14.740	74,069	13
L122	<u>2</u> /	<u>2</u> /	<u>2</u> /	<u>2</u> /	<u>2</u> /
L123	$\frac{\overline{2}}{2}$	<u>2</u> /	<u>2</u> /	<u>2</u> /	<u>2</u> /
L124	29,200	$5.\overline{3}40$	$0.\overline{433}$	14,842	54
L125	55,915	4.971	4.971	88,897	5
L126	43,707	5.061	5.061	115,791	1
Subtotal	5,956,439		251.980	(45,109)	
Total	6,558,054		<del>263.358</del>	43,899	
	, ,			(average)	

Based on Model inflow analysis. Included in the combined basin.

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#### Table A-8

#### TOTAL PEAK 1-YEAR/60-MINUTE WET-WEATHER FLOW

		2009			
	2009	<b>Basin Peak</b>	2009	2009	
	<b>Basin Peak</b>	Monitored	<b>Basin Peak</b>	<b>Basin Peak</b>	Wet
	<b>Hourly Flow</b>	infiltration	1-Year/60-Minute	Wet Weather	Weather
	Rate	(mgd)	Inflow	Flow	Peaking
Basin	(mgd)	12/24 Storm	(mgd)	(mgd)	Factor
Little Maumelle	Service Area			` •	
L113	0.888	0.516	3.088	4.492	8.7
L114	1.925	1.088	6.846	9.859	10.0
L116	0.678	$0.383^{1/}$	1.444	2.505	8.1
Subtotal	3.491	$\overline{1.987}$	11.378	16.856	
Adams & Fourc	<u>he Service Areas</u>				
L001&L002	1.322	0.401	6.543	8.267	11.7
L001	<u>2</u> /	<u>2</u> /	5.238	<u>2</u> /	<u>2</u> /
L002	<u>2</u> / <u>2</u> /	<u>2</u> / <u>2</u> /	1.306	<u>2</u> / <u>2</u> /	<u>2</u> / <u>2</u> /
L002R	0.113	1.169	0.774	2.056	29.0
L003	1.197	0.646	6.656	8.499	15.7
L003R	0.432	3.649	0.791	4.872	17.7
L004	0.333	0.329	2.188	2.849	15.2
$L004R^{3/}$	0.173	<u>3</u> /	0.905	1.078	8.5
L005	3.137	0.870	11.462	15.469	7.2
L005R	1.267	<u>4</u> /	2.108	3.375	3.8
L006	0.279	0.190	1.596	2.065	23.2
L007	0.919	0.764	6.043	7.726	23.7
L008	0.141	0.133	0.907	1.181	19.0
L008R	0.756	0.316	2.717	3.789	6.8
L009	0.588	0.239	2.215	3.042	9.8
L009R	0.936	<u>4</u> /	2.862	3.799	6.2
L010	0.802	$0.\overline{5}65$	7.786	9.153	19.8
L010R	4.793	1.922	16.467	23.182	7.5

- *Used alternate Infiltration period.*
- Included in combined meters.
- Bypass connection upstream precludes net infiltration period.
- Storage at Little Maumelle Lift Station precludes net infiltration calculation.
- Partial infiltration diverts South to Arch.
- Sewer line upstream was surcharged during Infiltration period.
- 1/ 2/ 3/ 4/ 5/ 6/ 7/ Additional flow diverted from L034 to a Lift Station most likely results in over estimate of the net Infiltration.
- Overflow upstream precludes net Infiltration calculation.
- Some flow from L009R is being diverted to a pump station so meter L122 and L123 infiltration may be an overestimate.

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#### TOTAL PEAK 1-YEAR/60-MINUTE WET-WEATHER FLOW

Basin	2009 Basin Peak Hourly Flow Rate (mgd)	2009 Basin Peak Monitored infiltration (mgd) 12/24 Storm	2009 Basin Peak 1-Year/60-Minute Inflow (mgd)	2009 Basin Peak Wet Weather Flow (mgd)	Wet Weather Peaking Factor
L012	0.275	0.275	2.911	3.461	19.3
L012 L014	1.122	0.490	4.404	6.016	11.0
L015	0.392	0.098	1.058	1.548	6.0
L018	0.509	0.635	1.942	3.086	10.2
L021	0.449	6/	0.515	0.964	3.2
L021 L023	0.185	0.192	2.036	2.413	18.1
L025	0.481	1.291	6.055	7.827	21.4
L026	0.075	0.196	0.528	0.799	14.5
L028	0.204	0.000	2.739	2.943	26.5
L029	0.386	0.435	4.081	4.902	26.6
L031	0.231	0.093	2.837	3.160	31.0
L032 <sup>7</sup> /	1.800	1.031	10.180	13.011	10.5
L034	2.096	1.082	13.097	16.275	12.8
L035	0.369	0.387	1.941	2.697	10.1
L036	0.220	0.297	1.199	1.715	10.7
L037	0.696	0.156	5.055	5.908	17.6
L043	0.883	0.569	3.082	4.534	10.0
L044	0.794	0.108	2.659	3.561	6.9
L050	1.159	<u>6</u> /	5.202	6.361	8.6
L055	1.007	< 0.001	1.931	2.938	4.8
L057	0.194	0.049	1.106	1.349	14.2
L058	1.246	0.459	4.742	6.447	8.3
L059	0.131	0.571	3.269	3.971	59.3
L062	0.274	0.464	1.336	2.074	16.6
L063	0.487	0.394	4.150	5.030	15.3
L100	0.573	0.573	6.860	8.005	21.5

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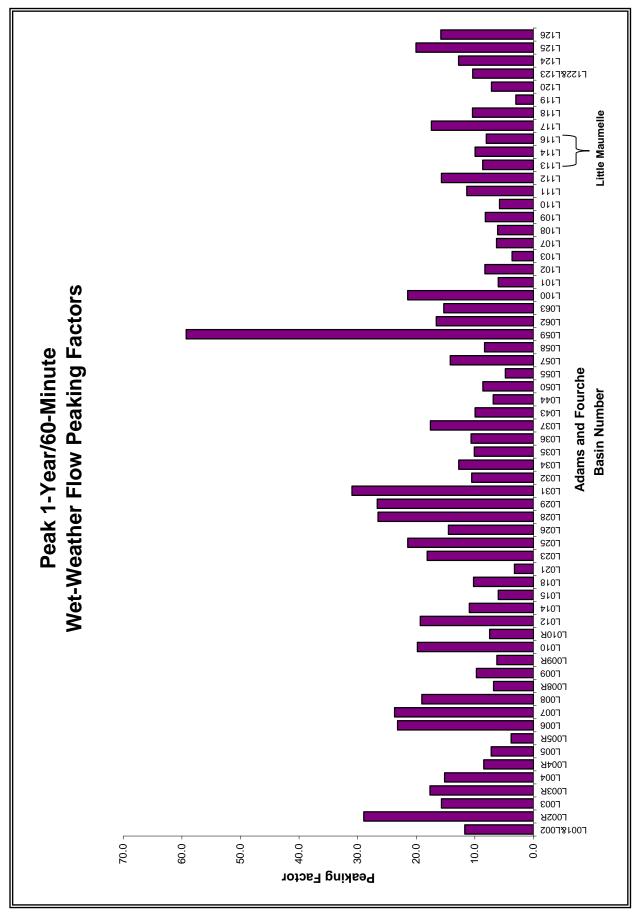
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#### TOTAL PEAK 1-YEAR/60-MINUTE WET-WEATHER FLOW

	2009 Basin Peak Hourly Flow Rate	2009 Basin Peak Monitored infiltration (mgd)	2009 Basin Peak 1-Year/60-Minute Inflow	2009 Basin Peak Wet Weather Flow	Wet Weather Peaking
Basin	(mgd)	12/24 Storm	(mgd)	(mgd)	Factor
L101	0.806	0.180	1.598	2.584	6.0
L102	2.059	<u>2</u> /	6.539	9.298	8.3
L103	0.732	<u>2</u> / <u>8</u> / <u>2</u> /	1.368	2.099	3.6
L105	<u>2</u> /	<u>2</u> /	0.712	0.712	<u>2</u> /
L107	0.893	0.376	2.661	3.930	6.3
L108	0.541	0.167	1.639	2.347	6.1
L109	0.367	0.148	1.305	1.821	8.2
L110	0.419	0.173	0.911	1.503	5.8
L111	0.180	0.078	1.052	1.310	11.4
L112	3.873	1.763	32.234	37.869	15.7
L117	0.668	0.351	6.503	7.522	17.4
L118	0.447	0.236	1.860	2.543	10.4
L119	0.344	0.077	0.235	0.655	3.0
L120	0.699	0.375	1.222	2.296	7.2
L122&L123 <sup>9/</sup>	3.236	6.023	14.740	23.999	10.4
L122	<u>2</u> /	<u>2</u> /	3.745	<u>2</u> /	<u>2</u> /
L123	<u>2</u> / <u>2</u> /	<u>2</u> / <u>2</u> /	10.995	<u>2</u> / <u>2</u> /	<u>2</u> / <u>2</u> /
L124	0.206	1.304	0.433	1.943	12.8
L125	0.610	0.676	4.971	6.257	20.1
L126	0.668	0.405	5.061	6.134	<u>15.8</u>
Subtotal	<u>50.172</u>	<u>34.713</u>	<u>251.980</u>	<u>336.865</u>	
Total	53.663	36.700	263.358	353.721	13.5
					(average)

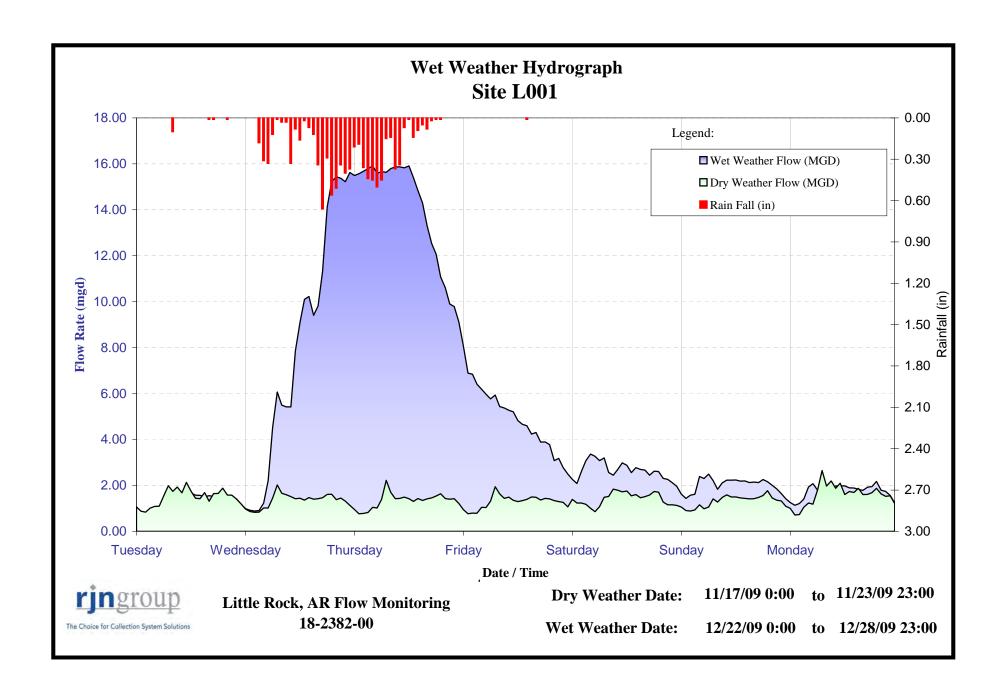
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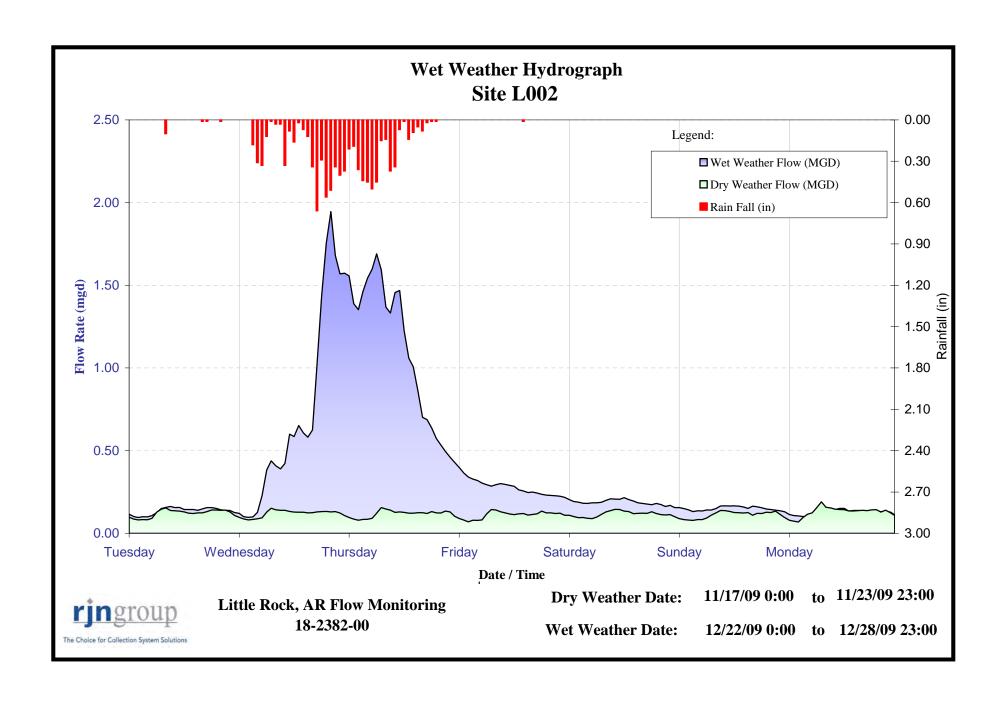
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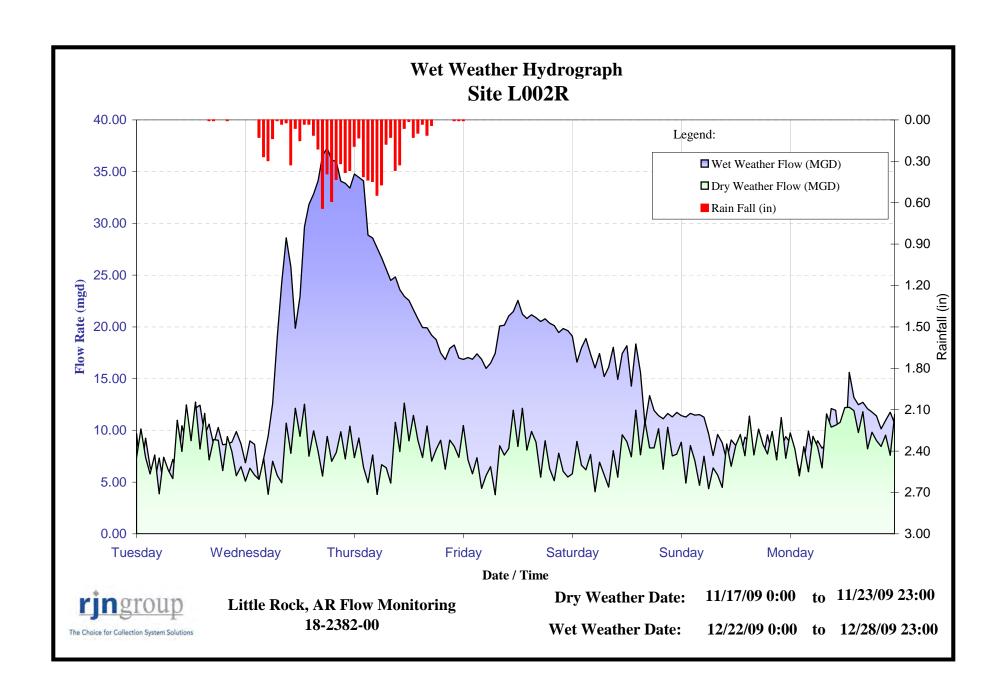


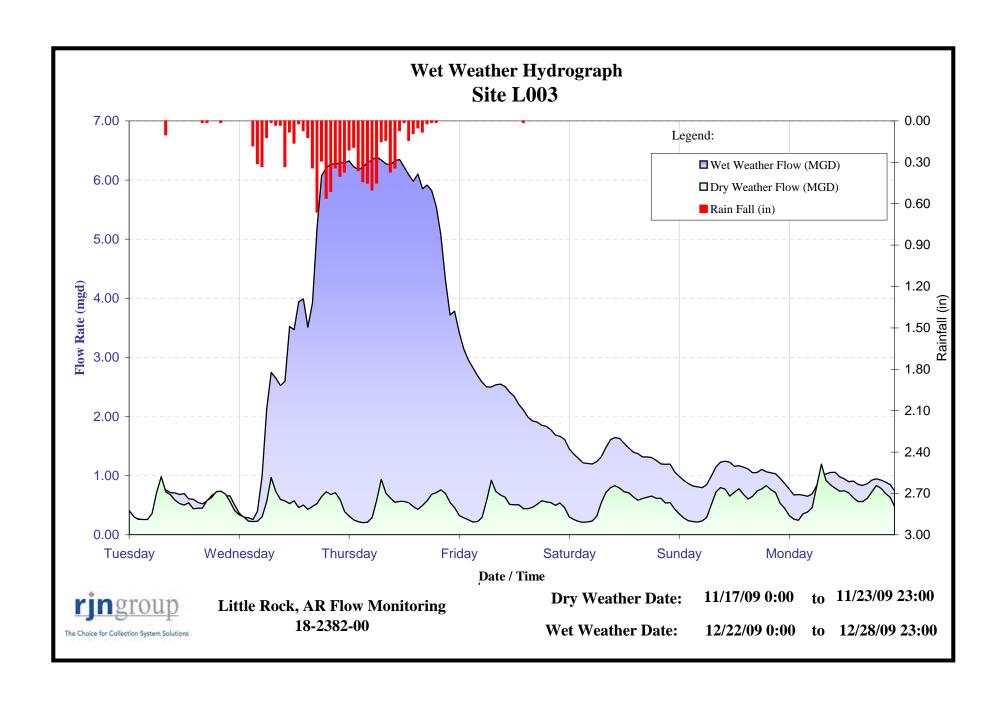
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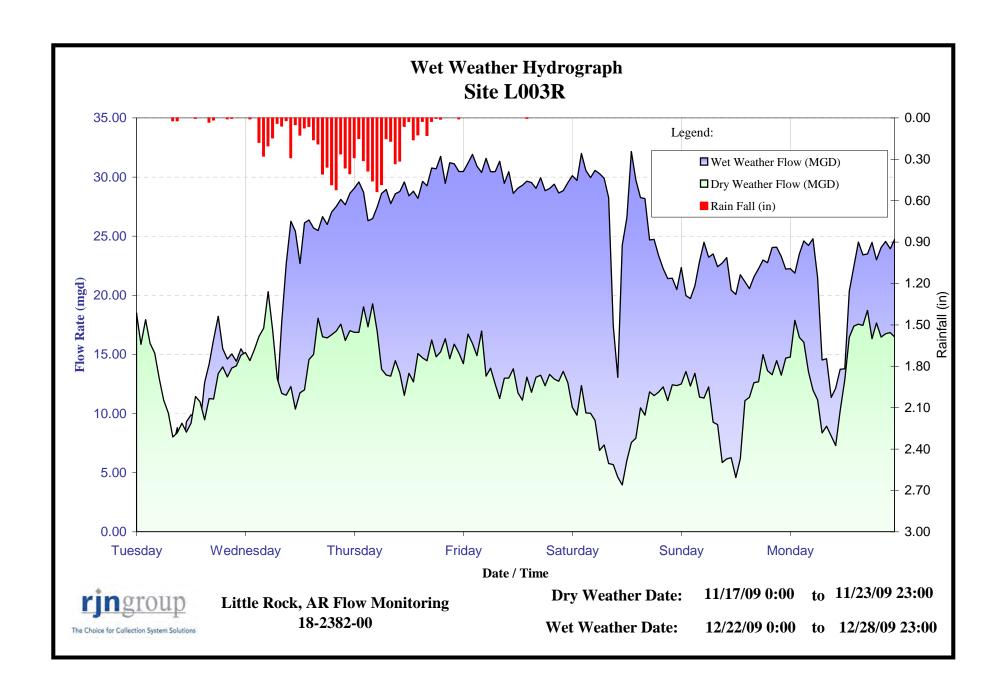
# APPENDIX A WET VS. DRY HYDROGRAPHS

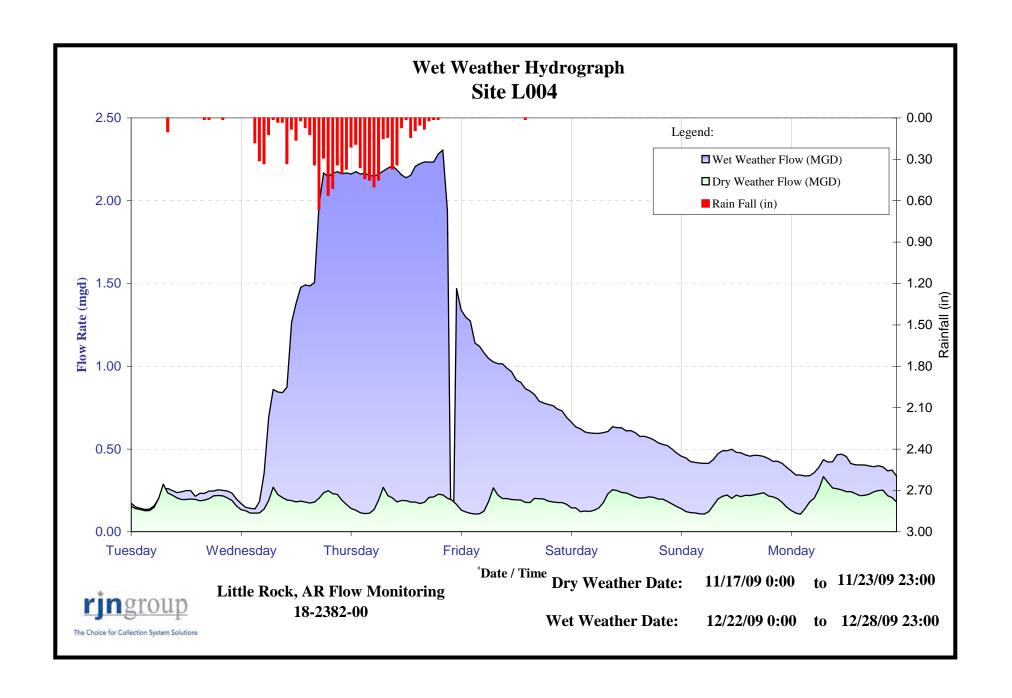


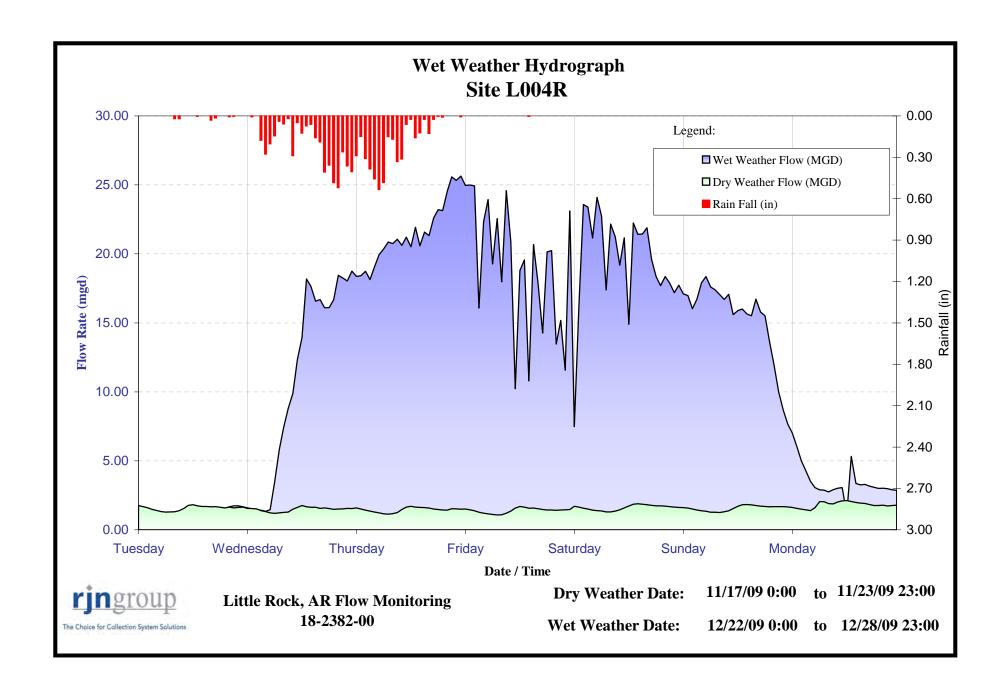


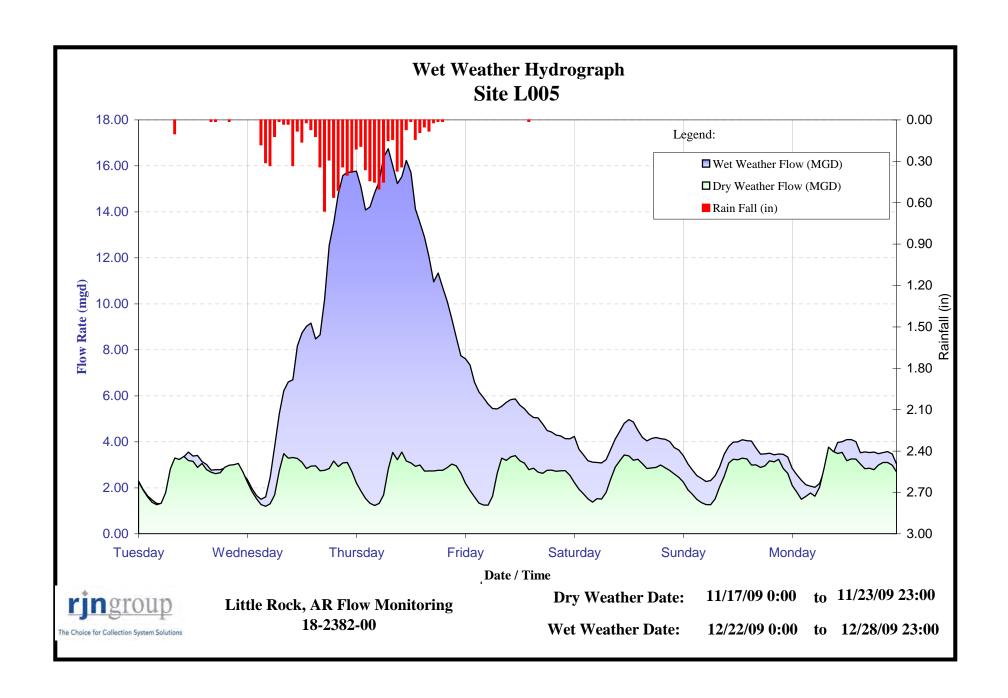


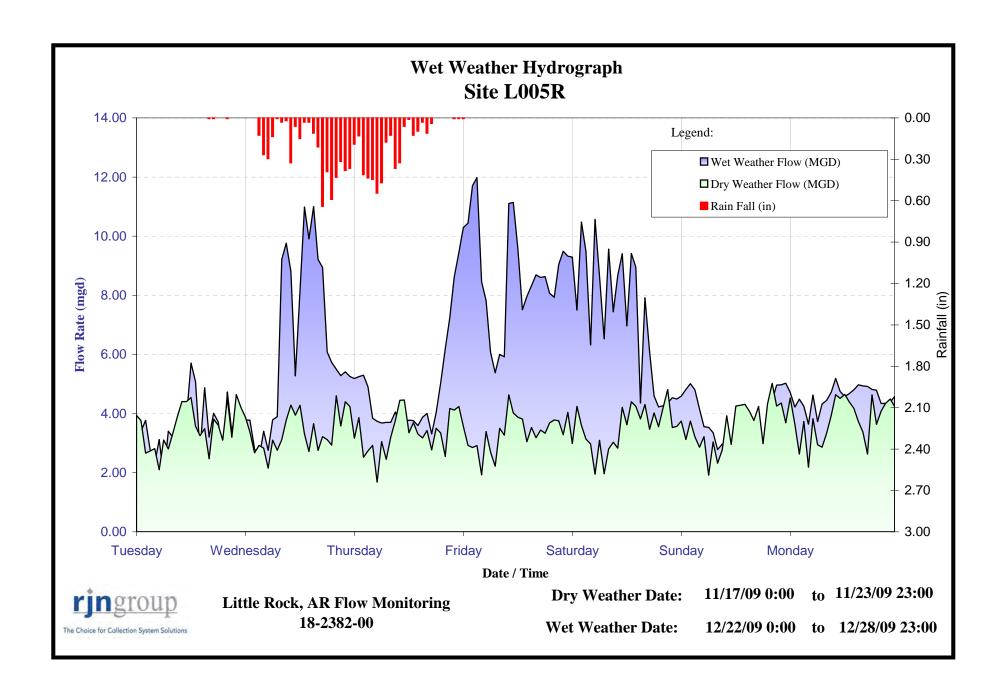


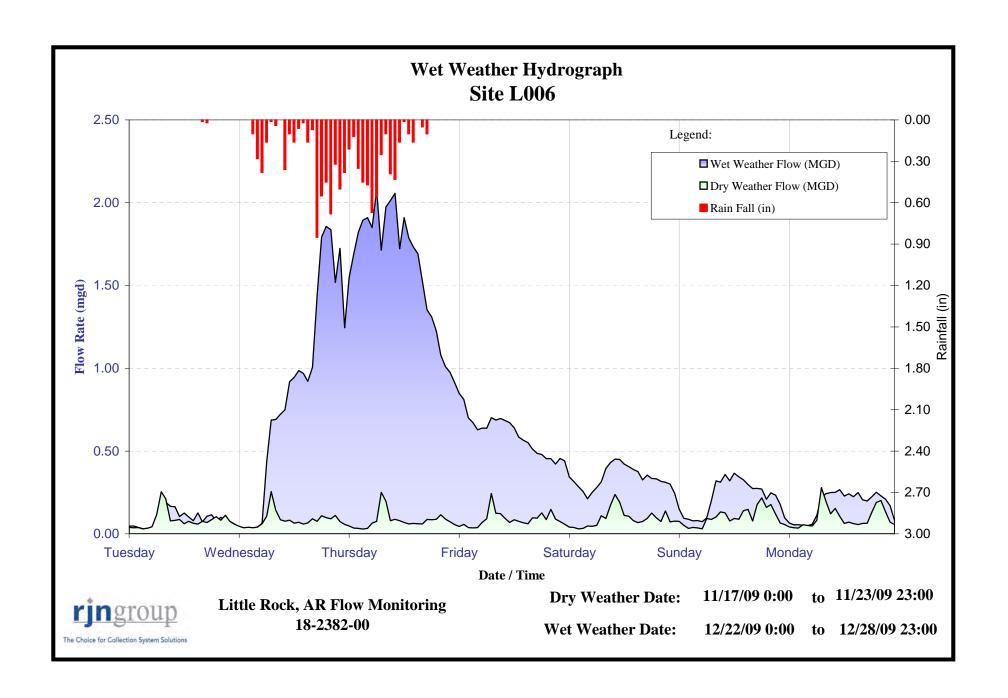


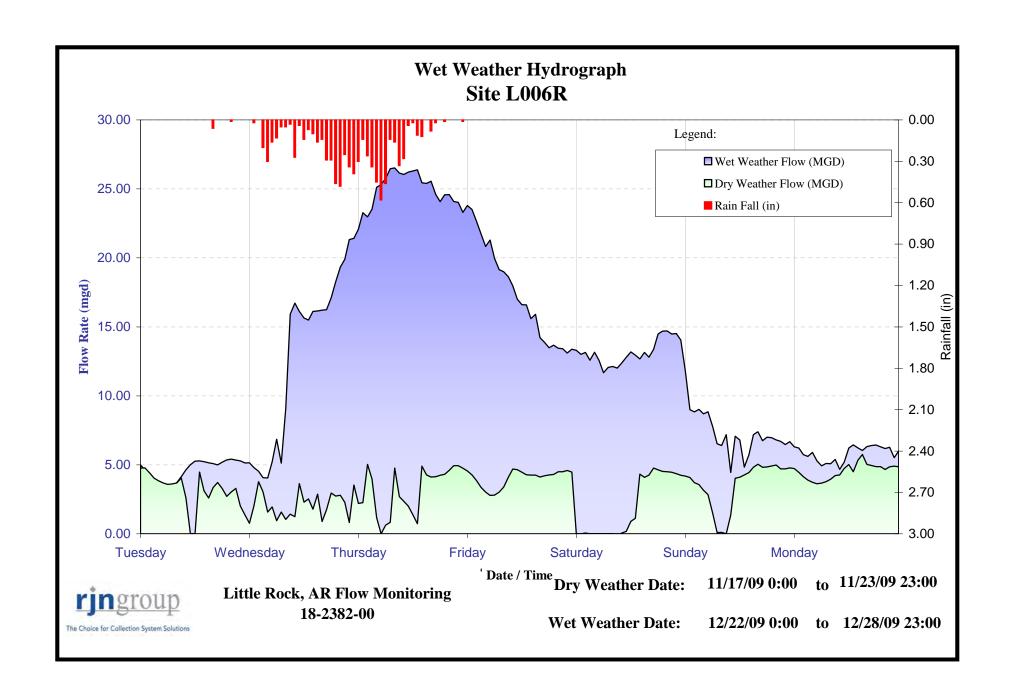


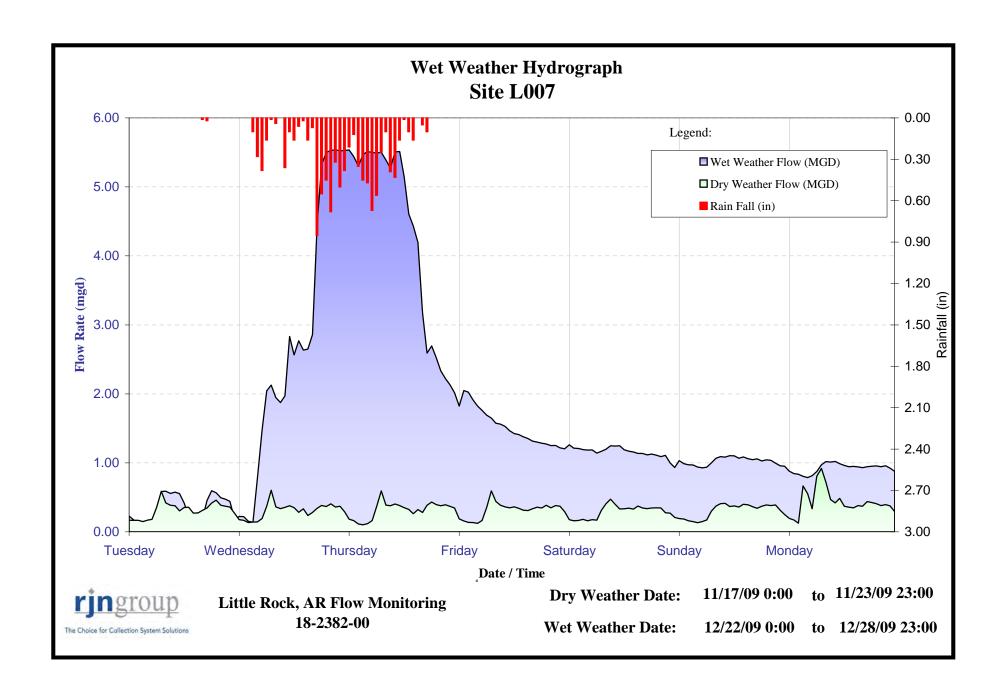


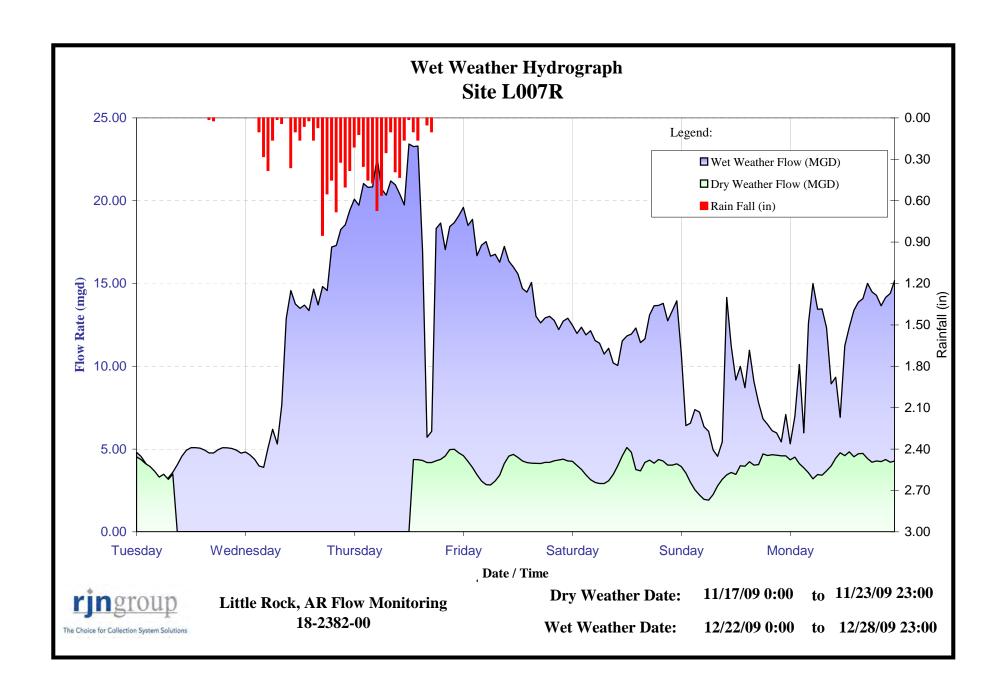


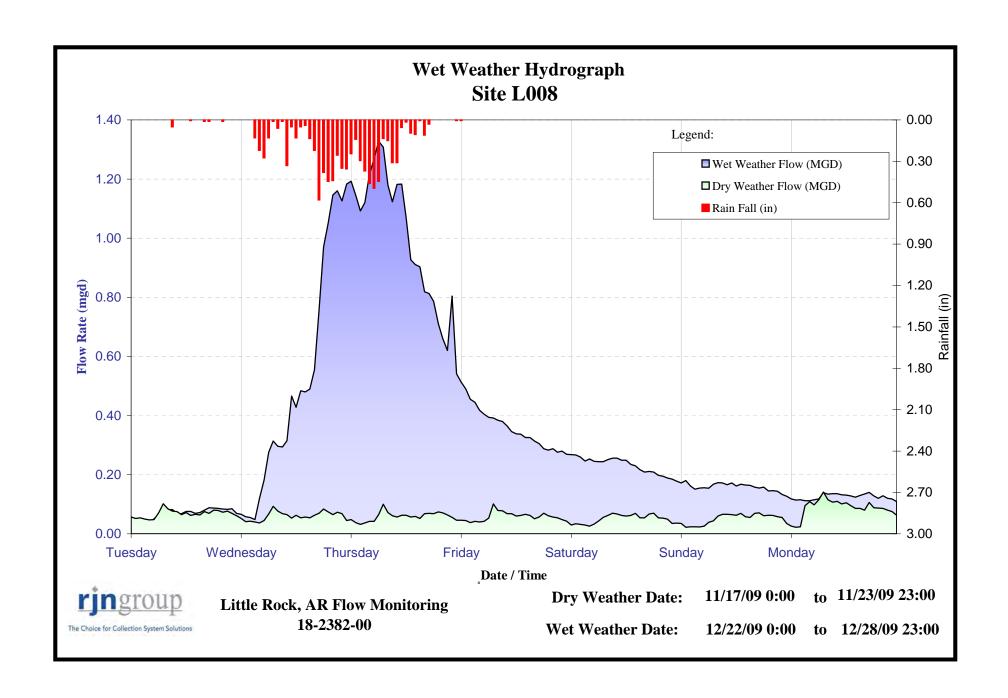


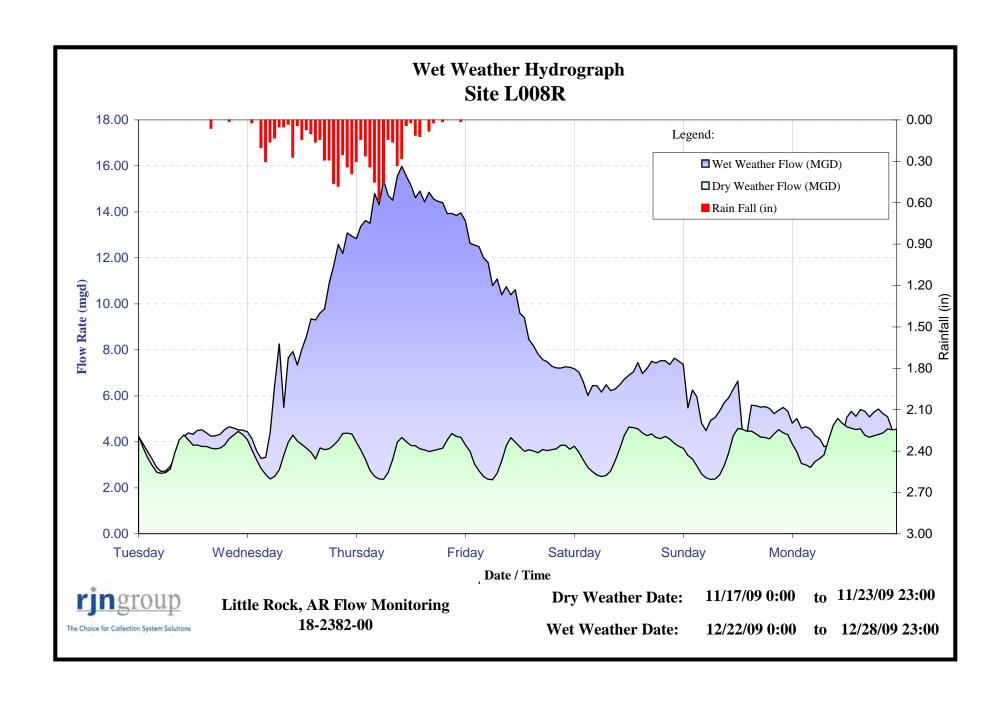


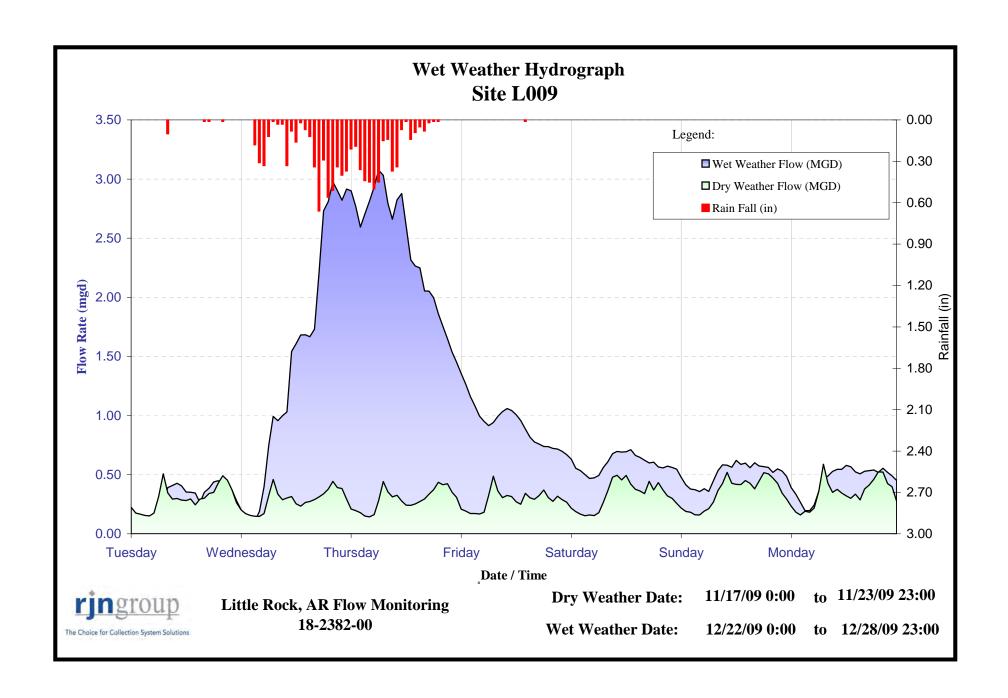


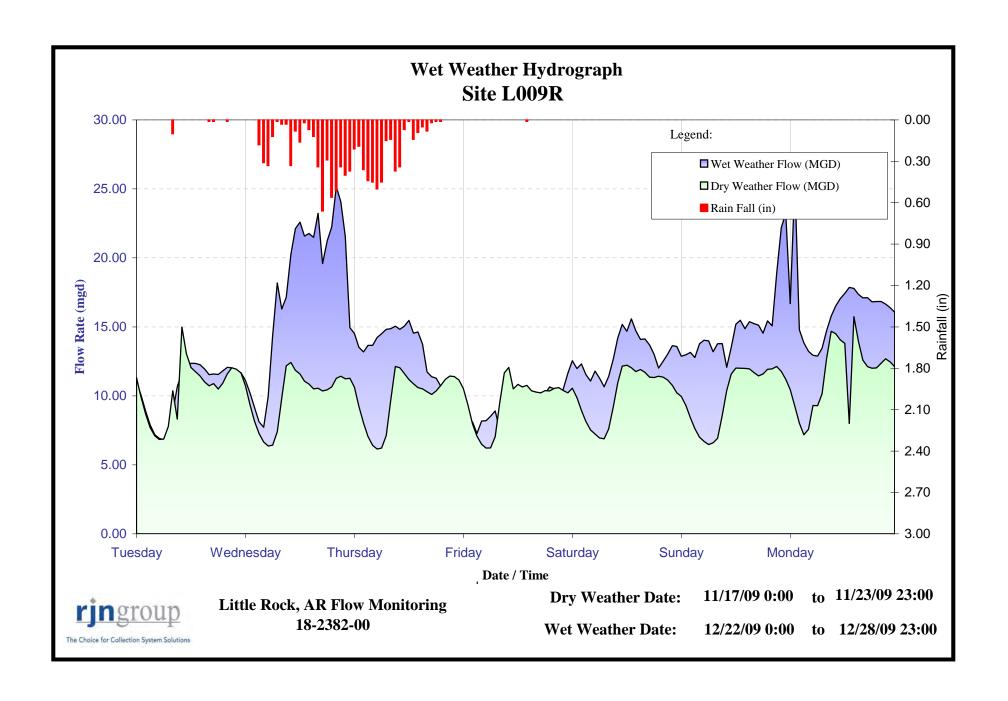


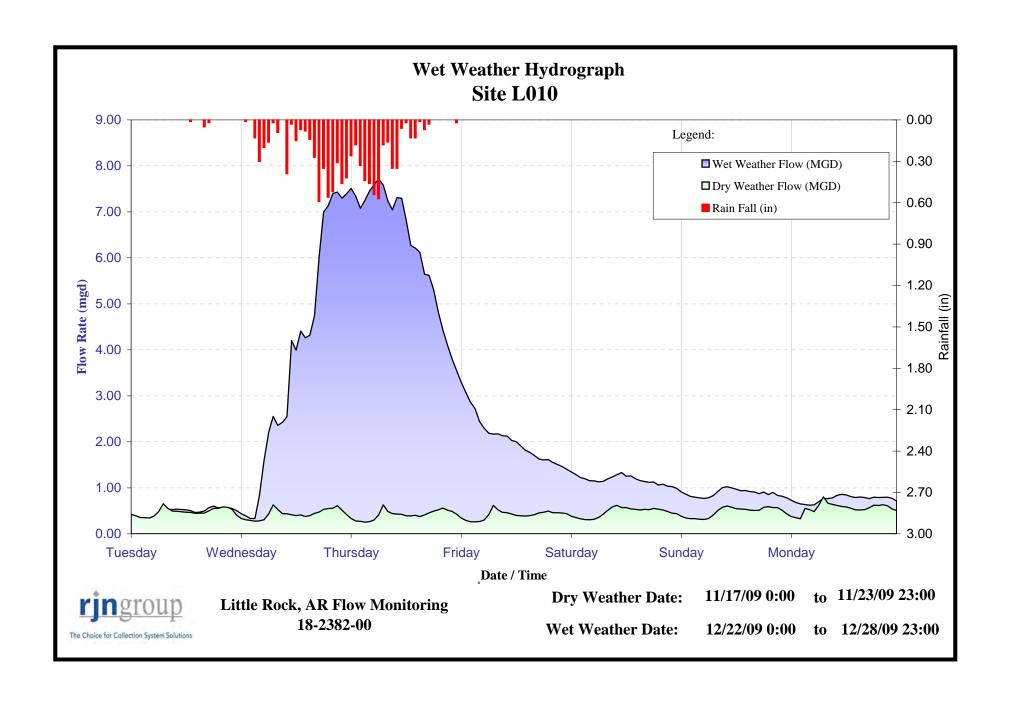


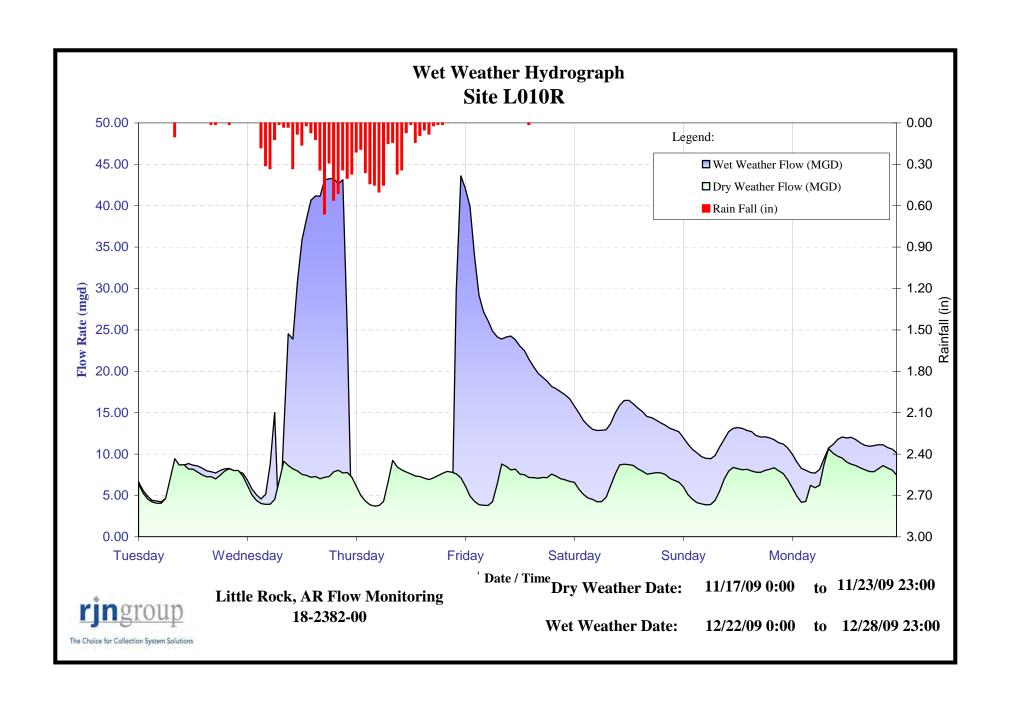


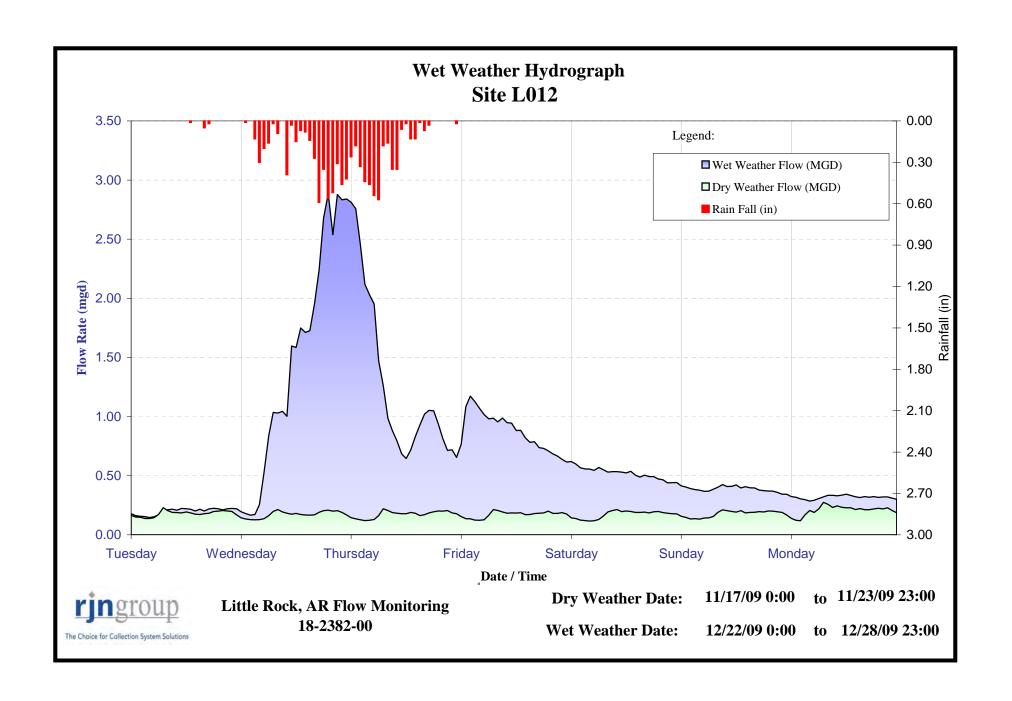


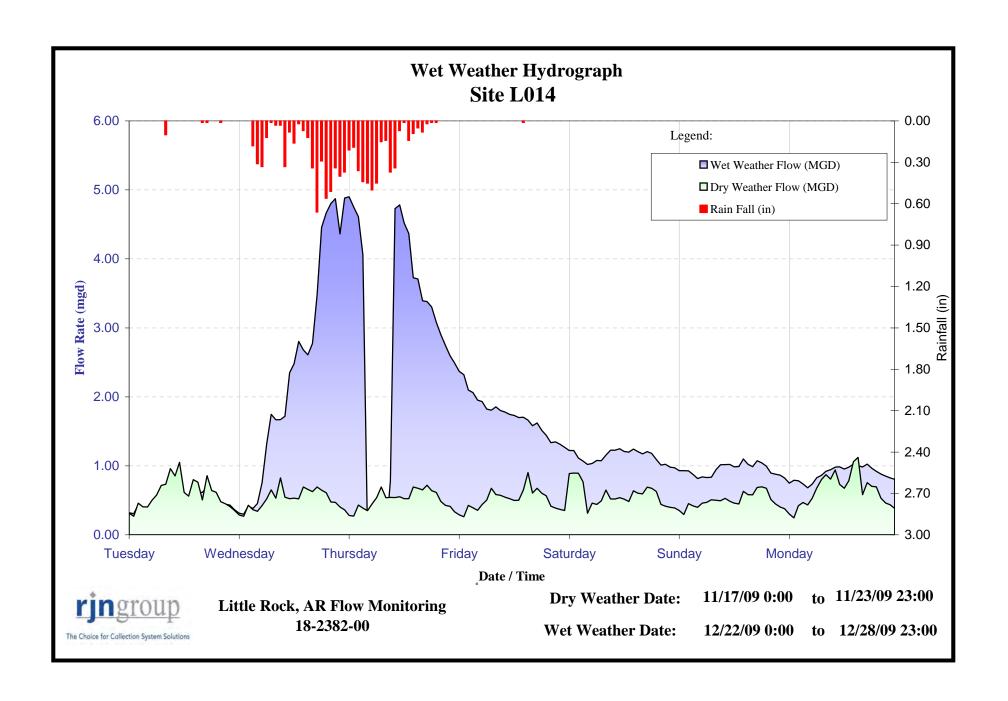


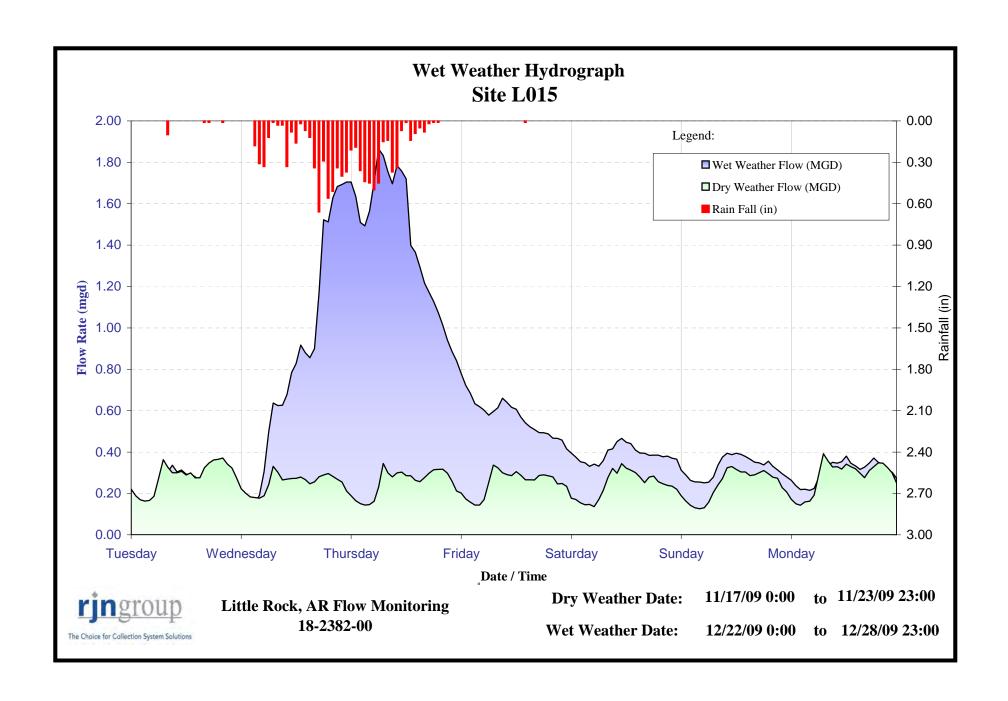


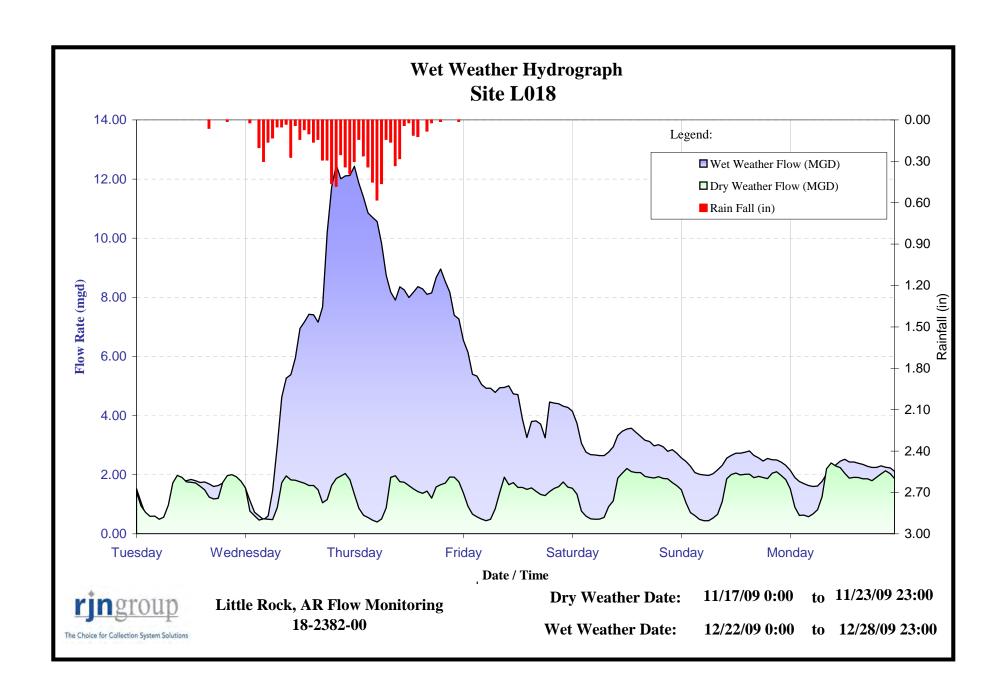


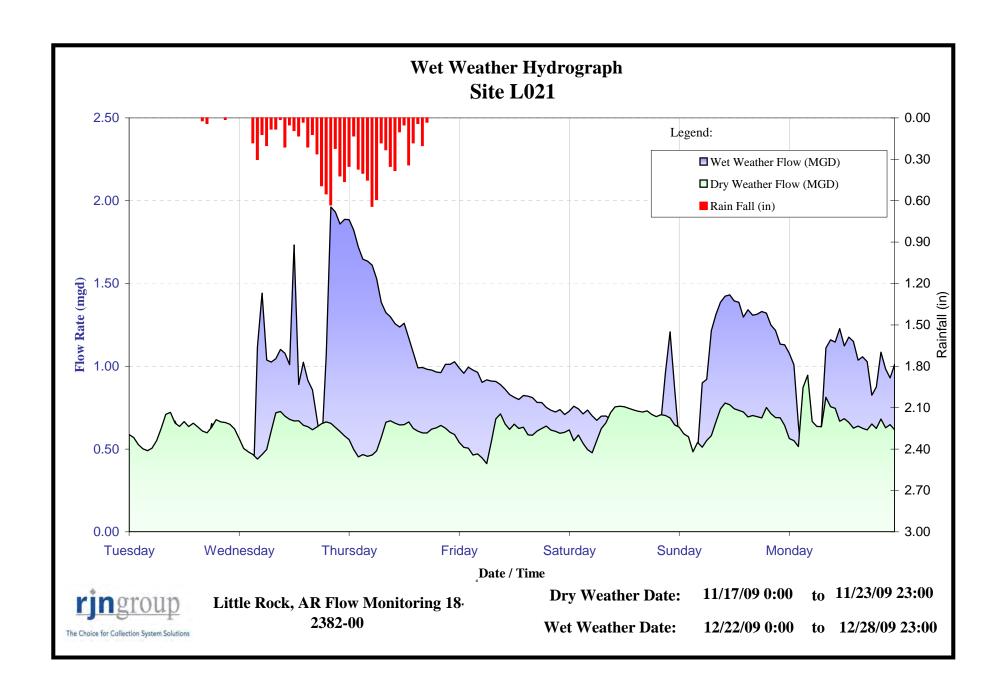


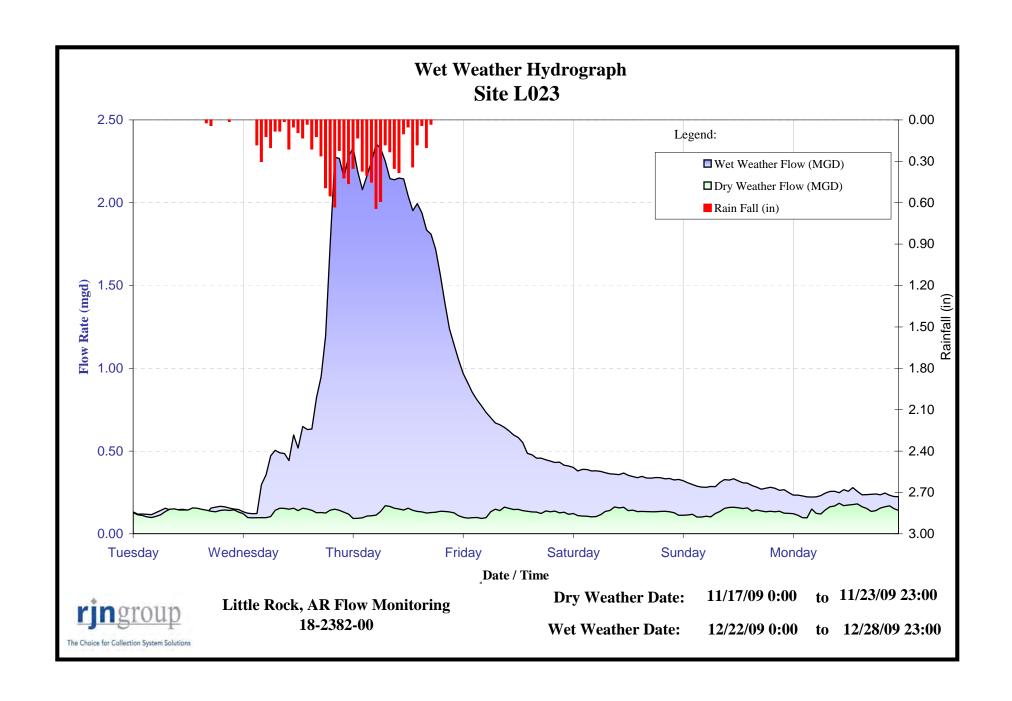


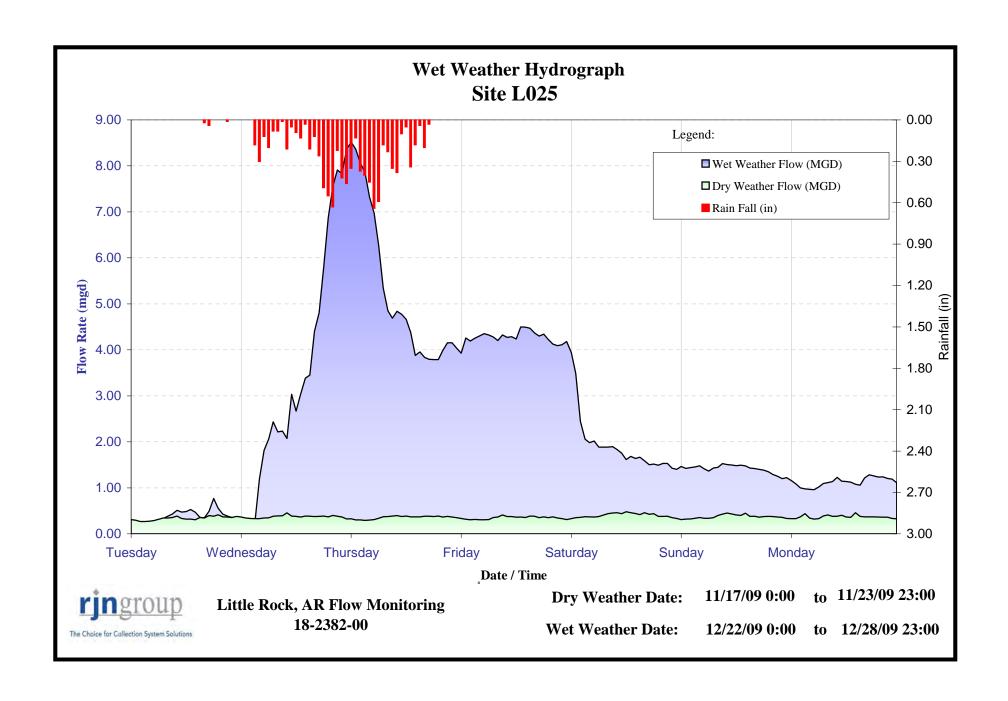


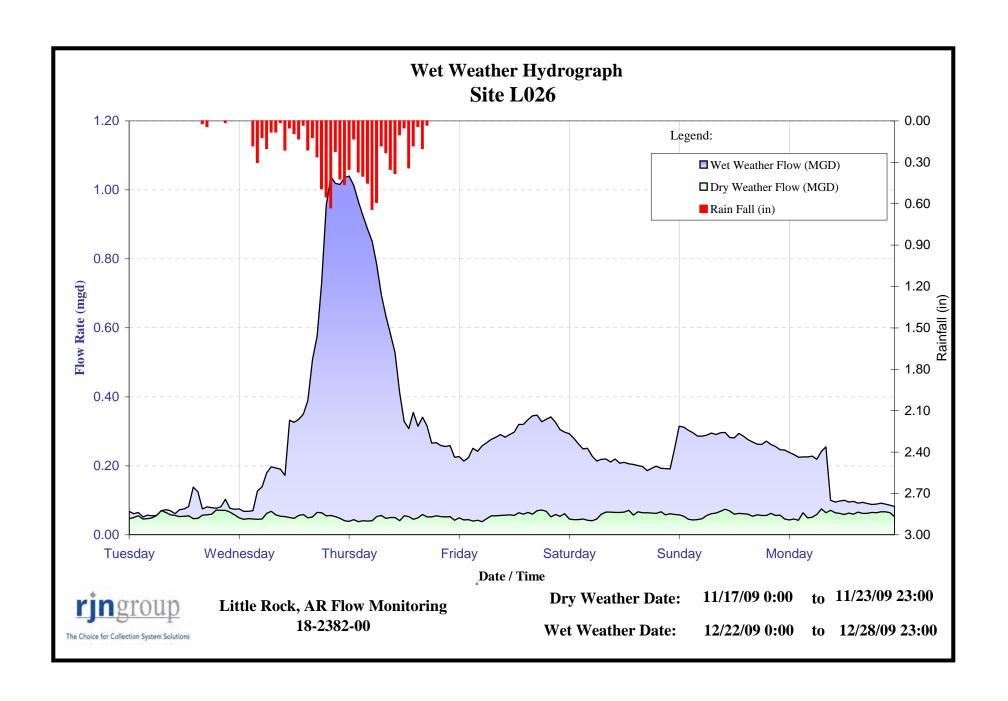


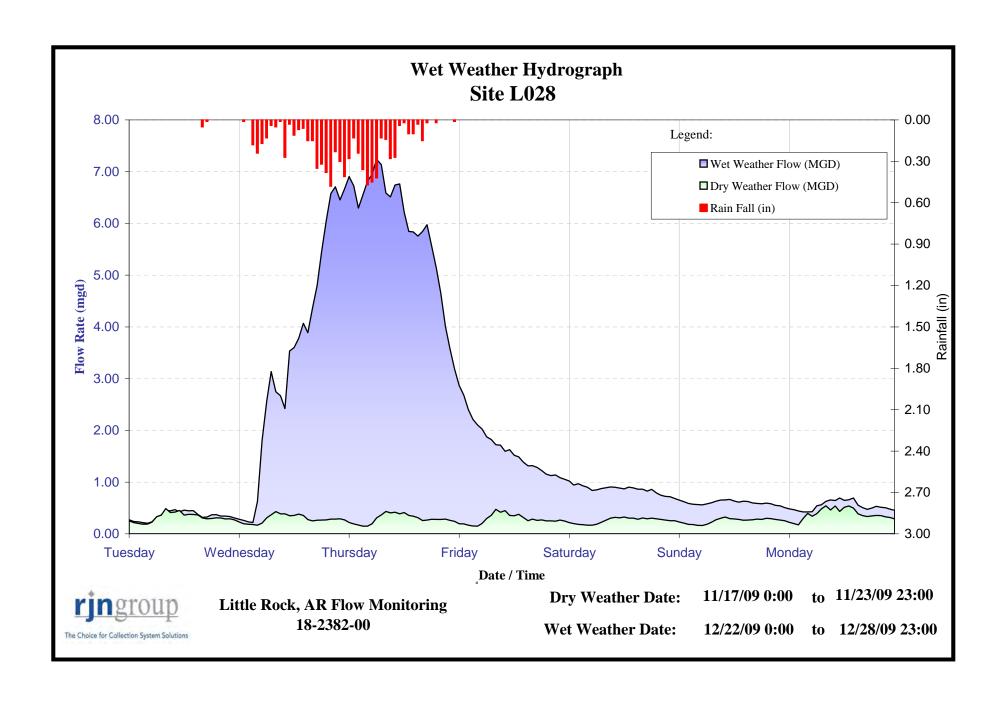


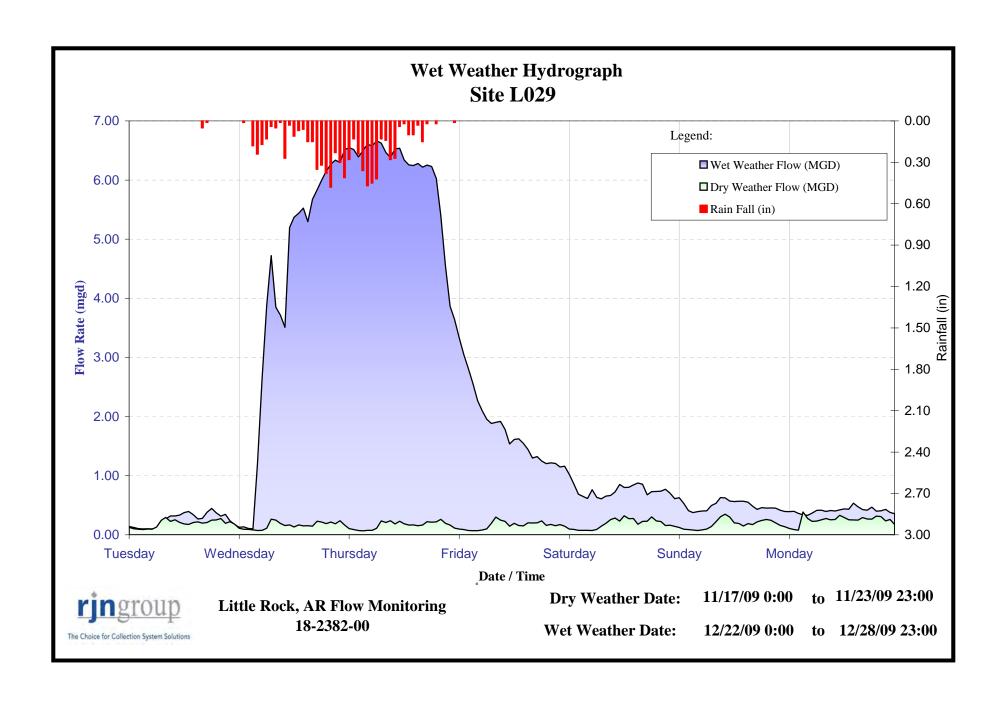


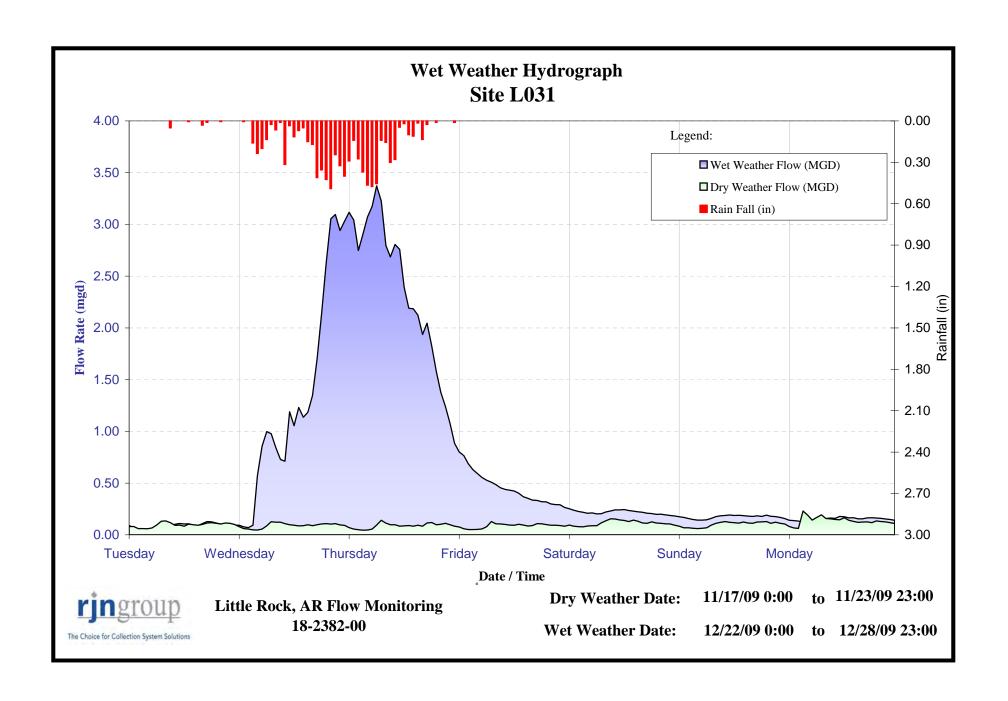


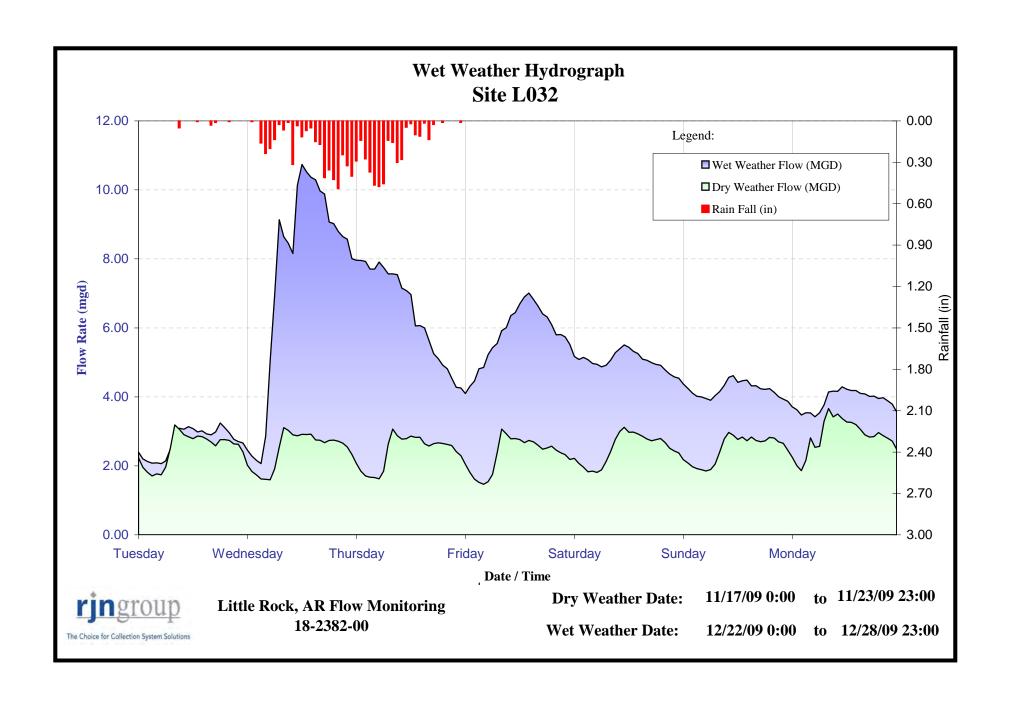


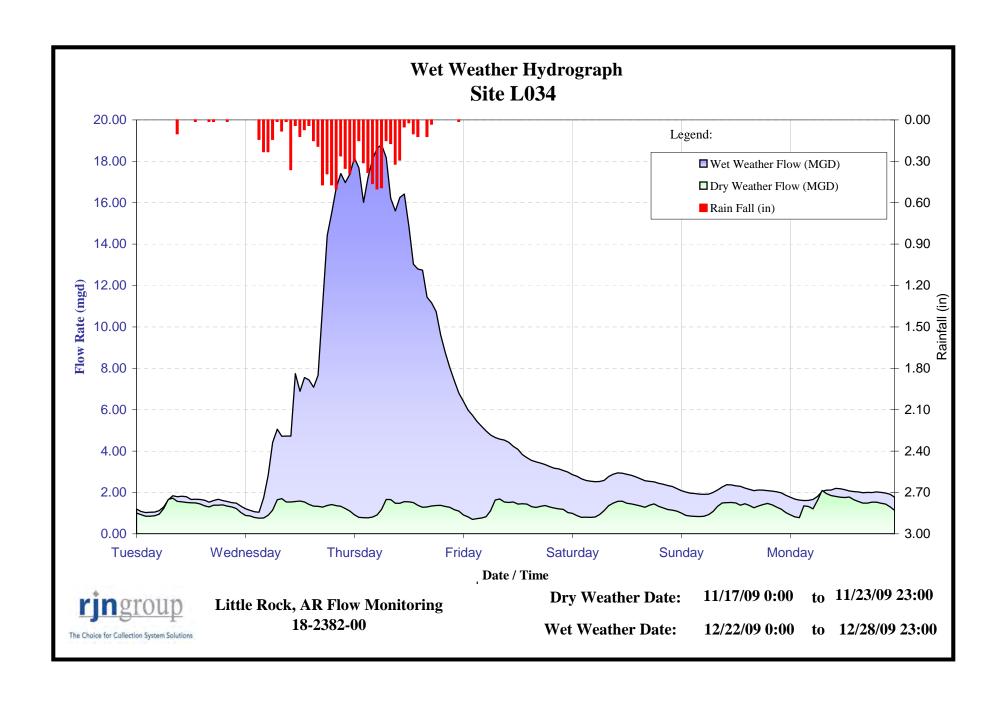


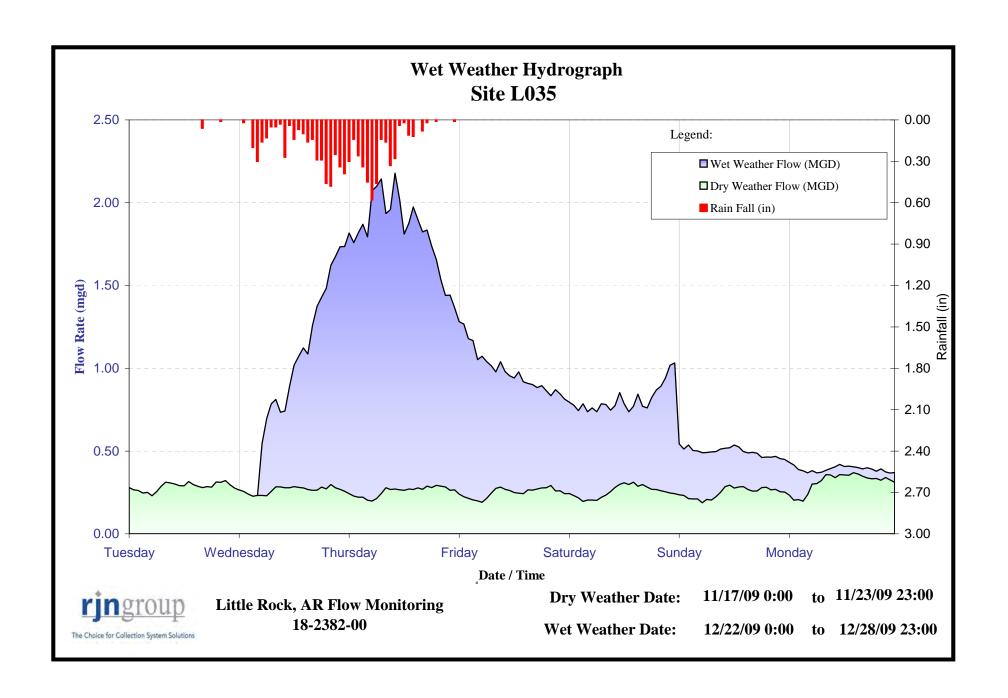


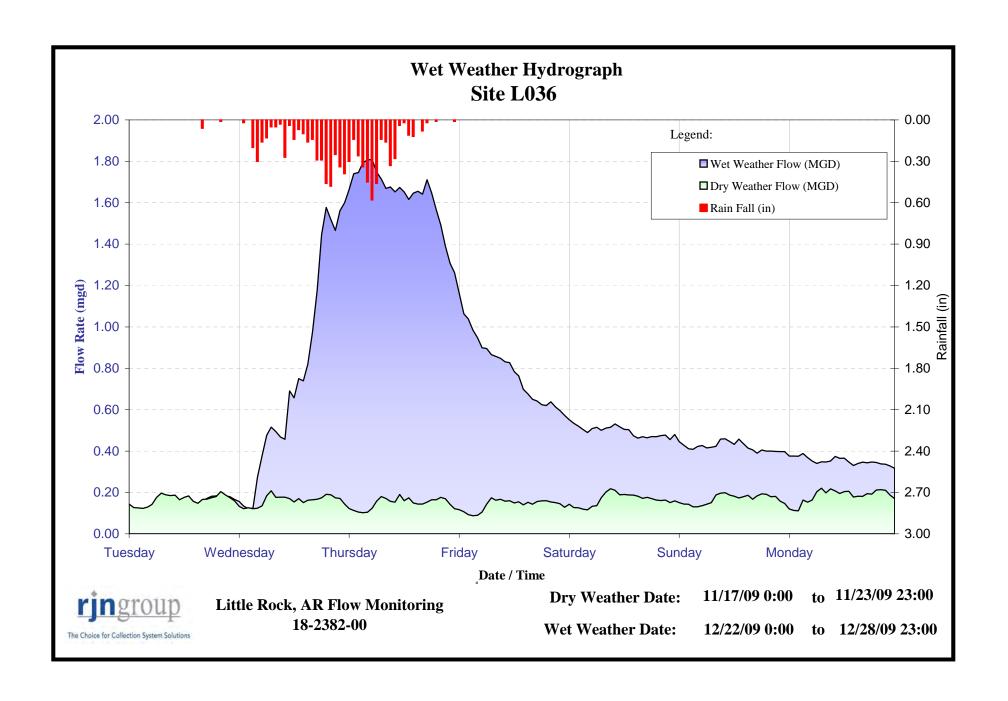


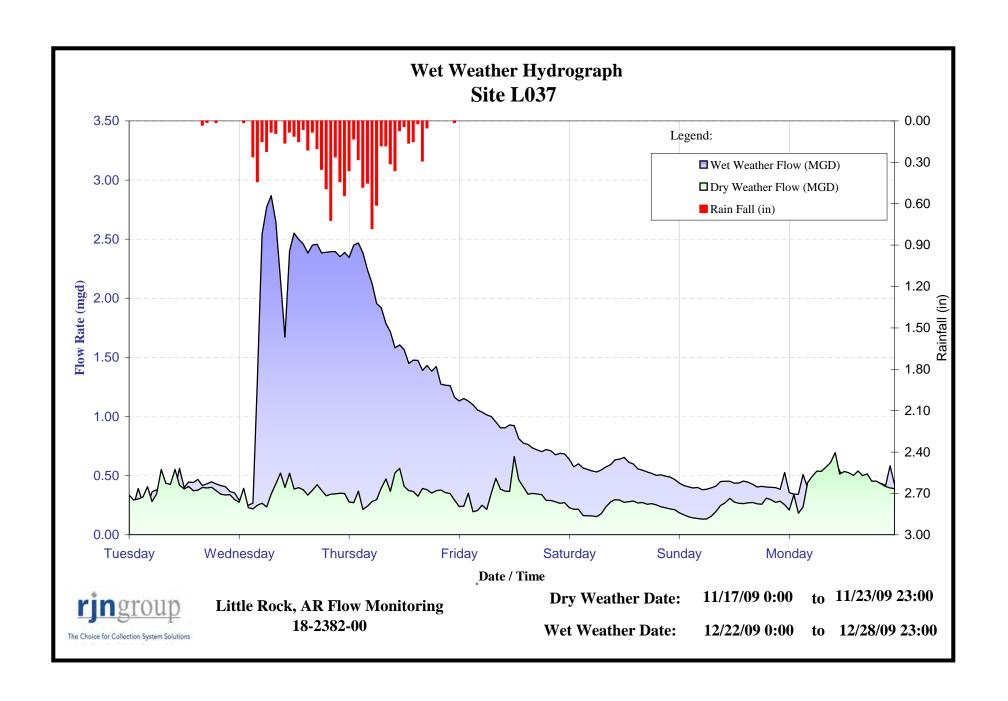


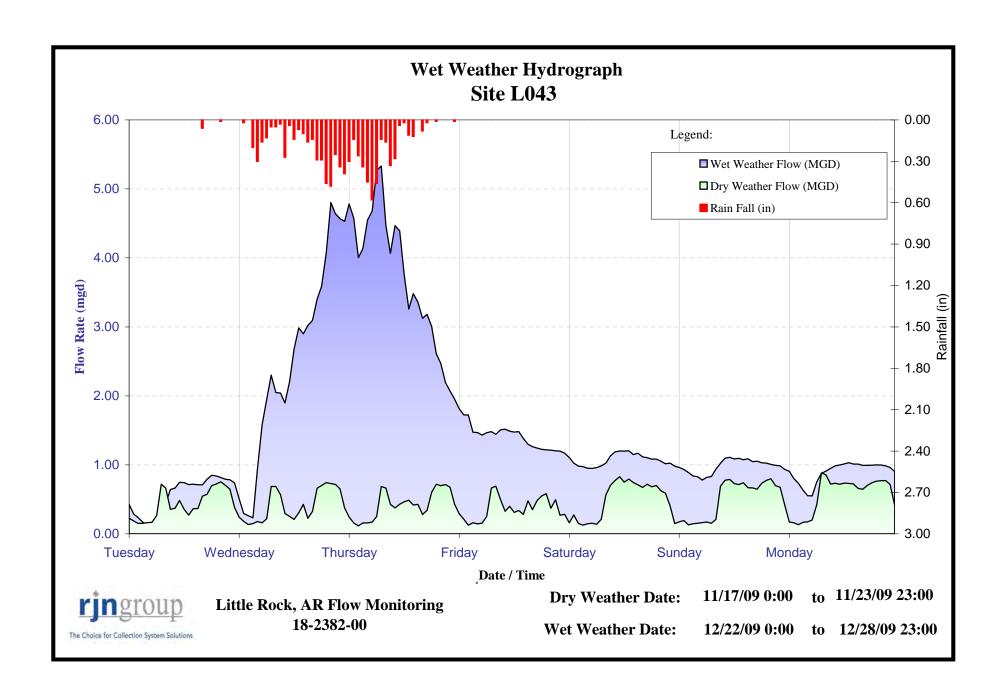


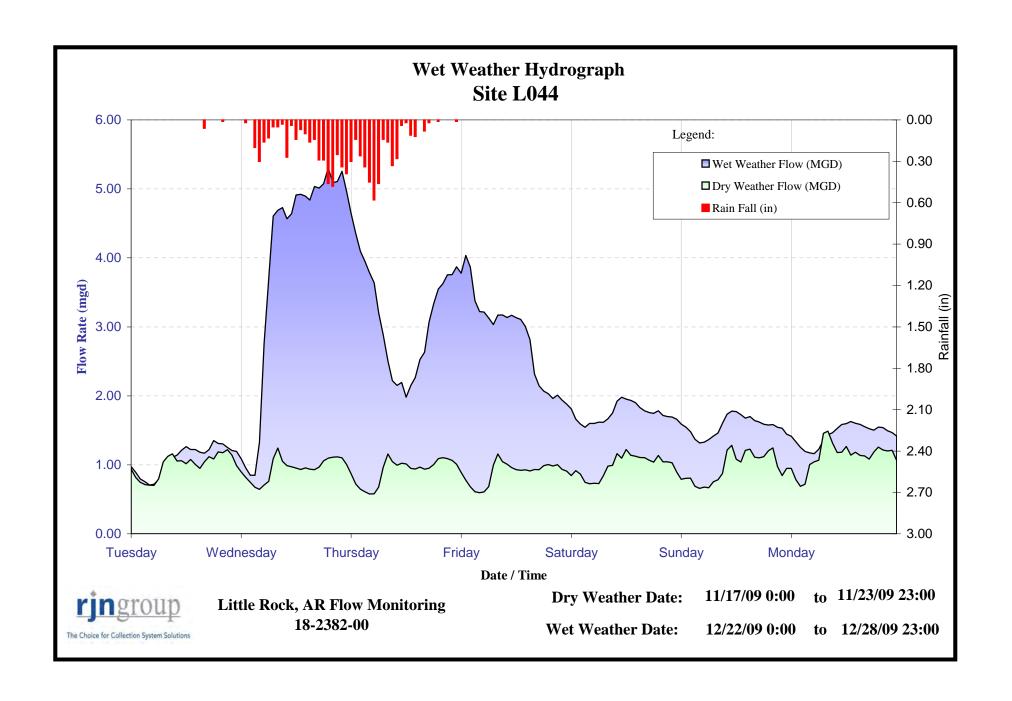


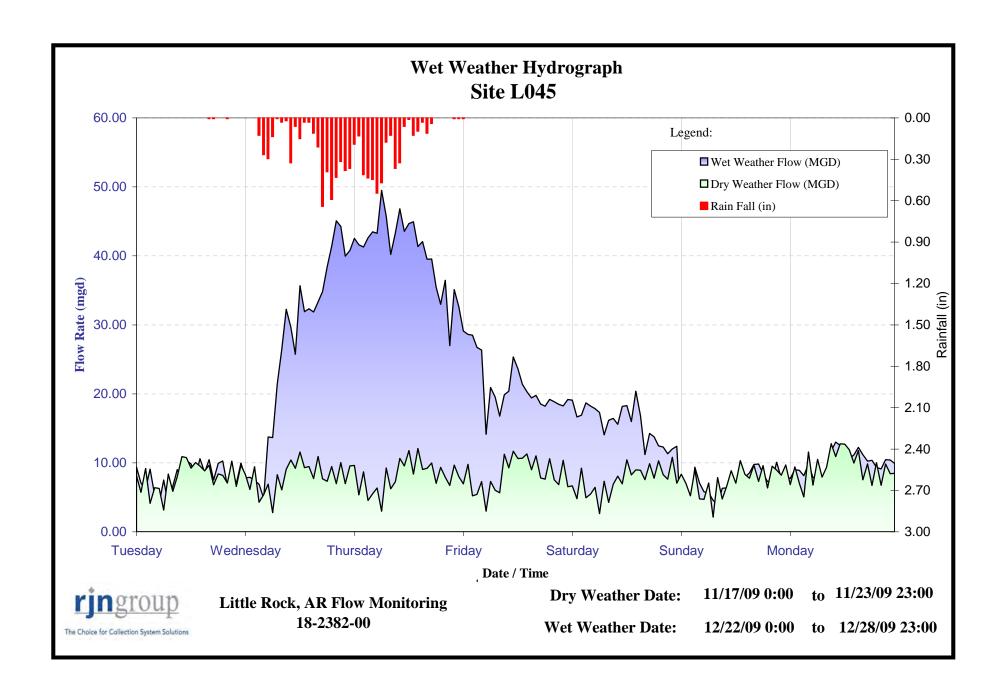


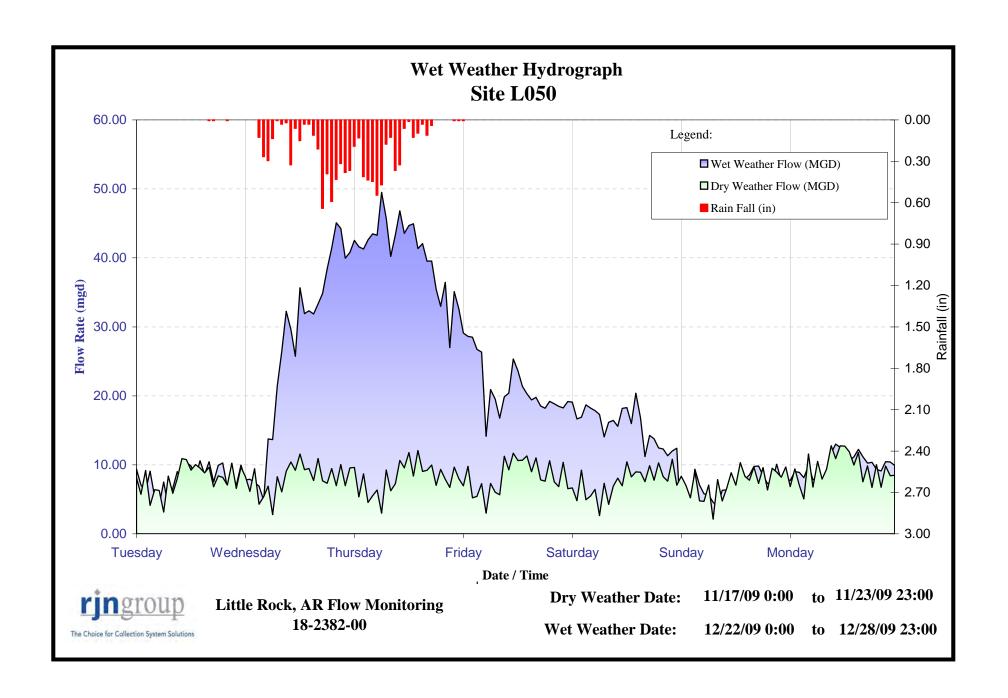


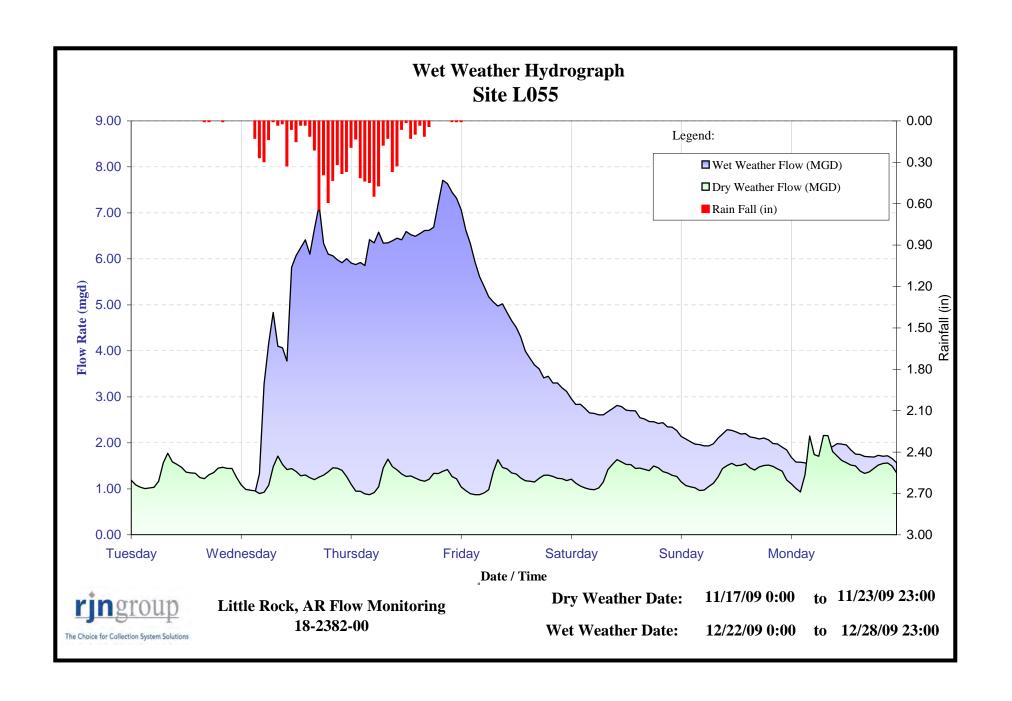


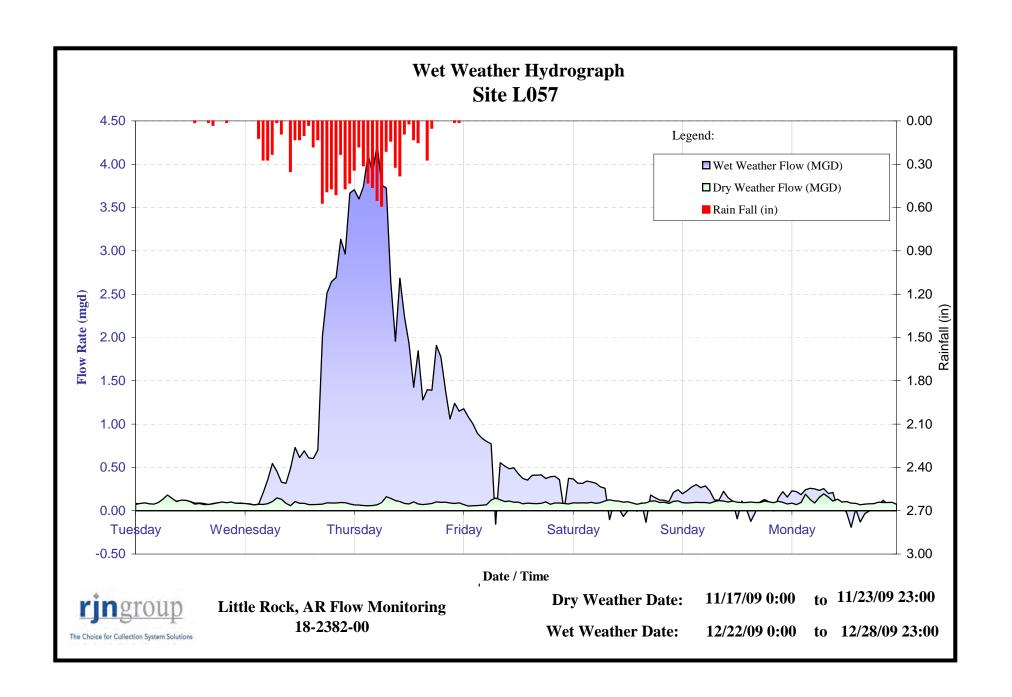


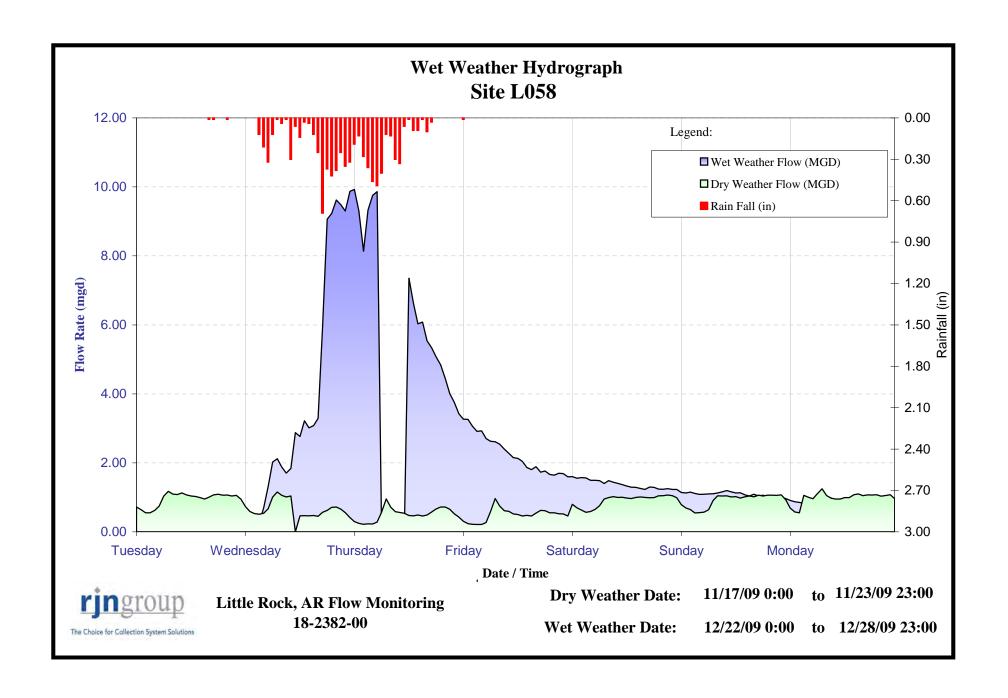


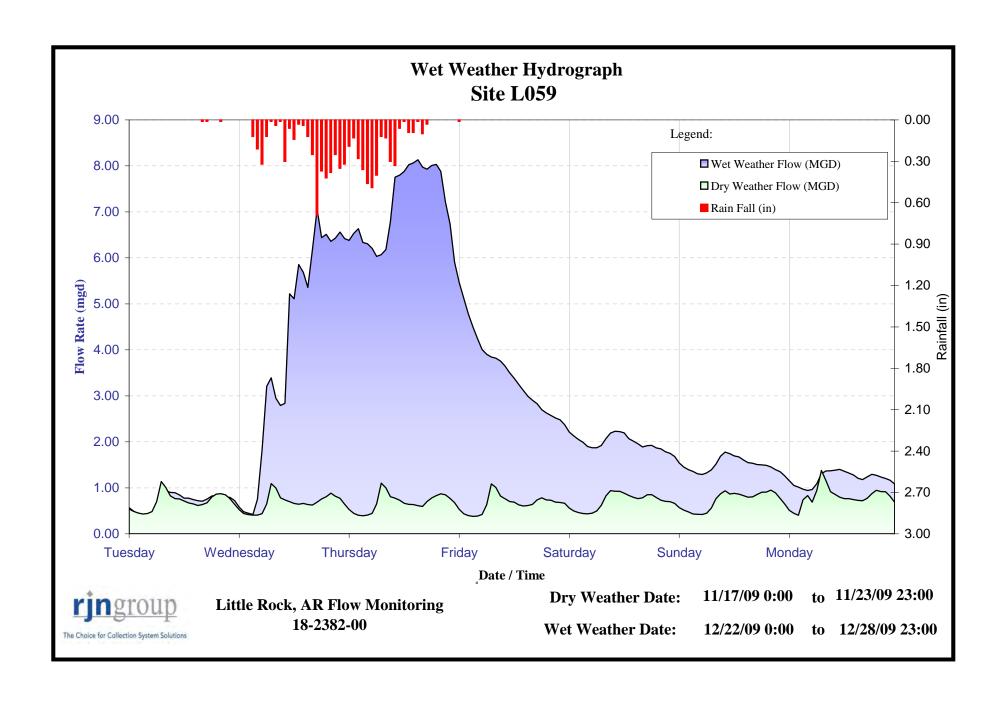


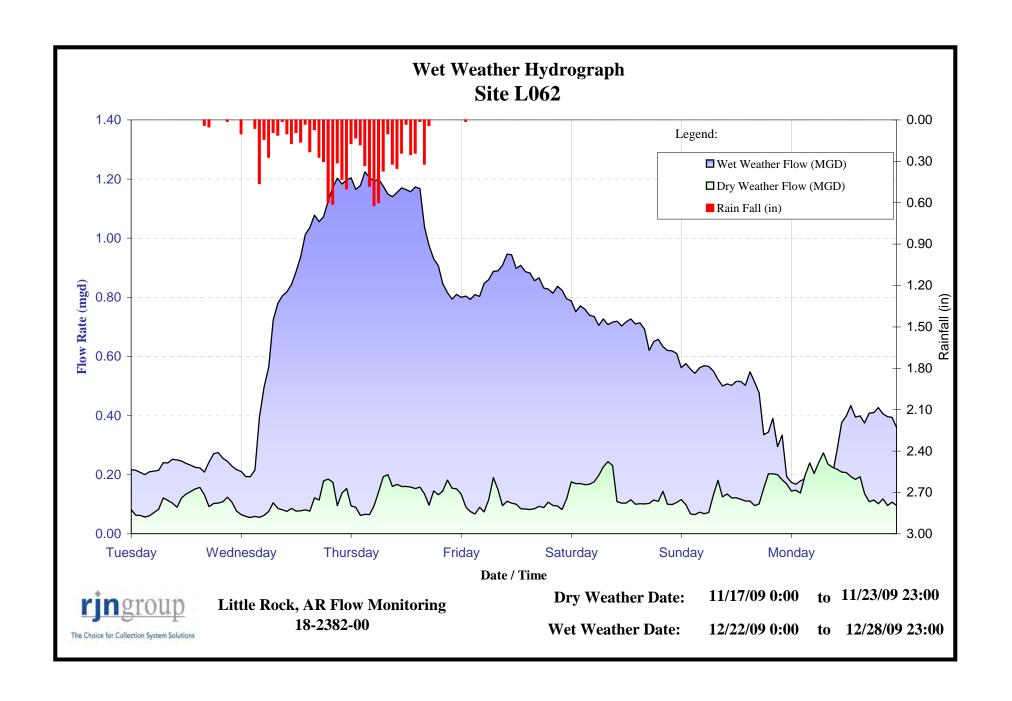


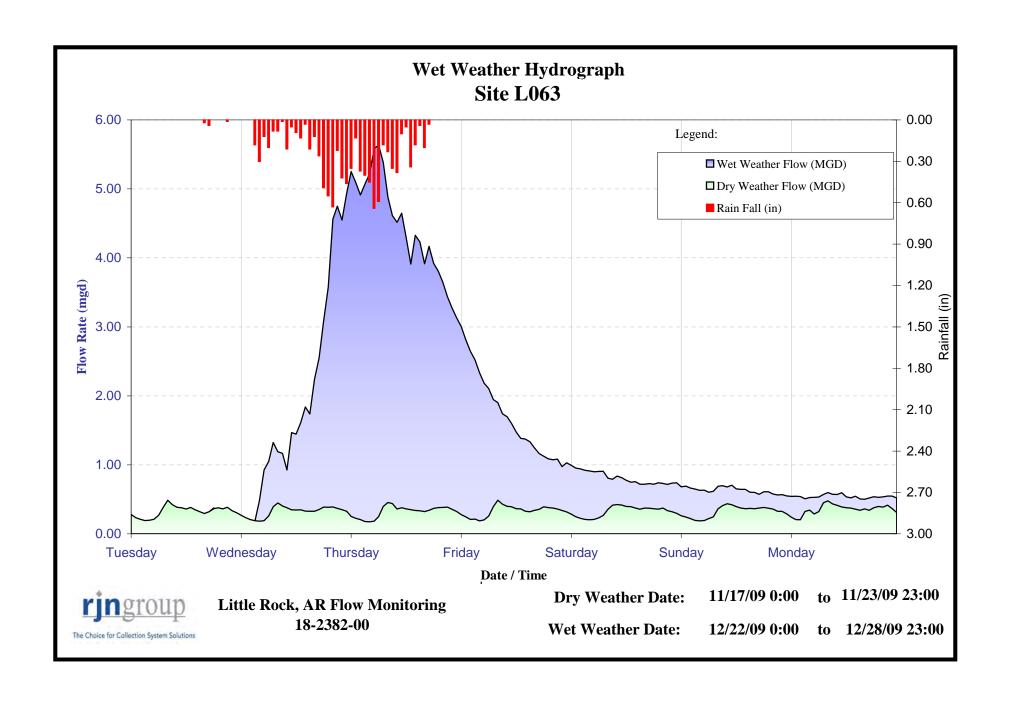


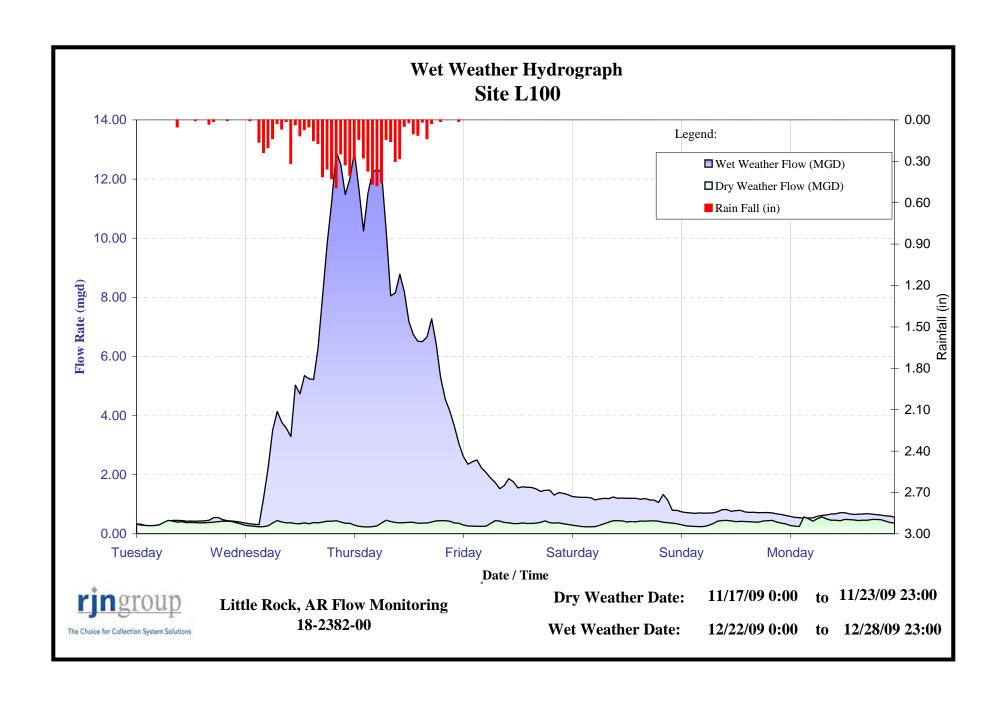


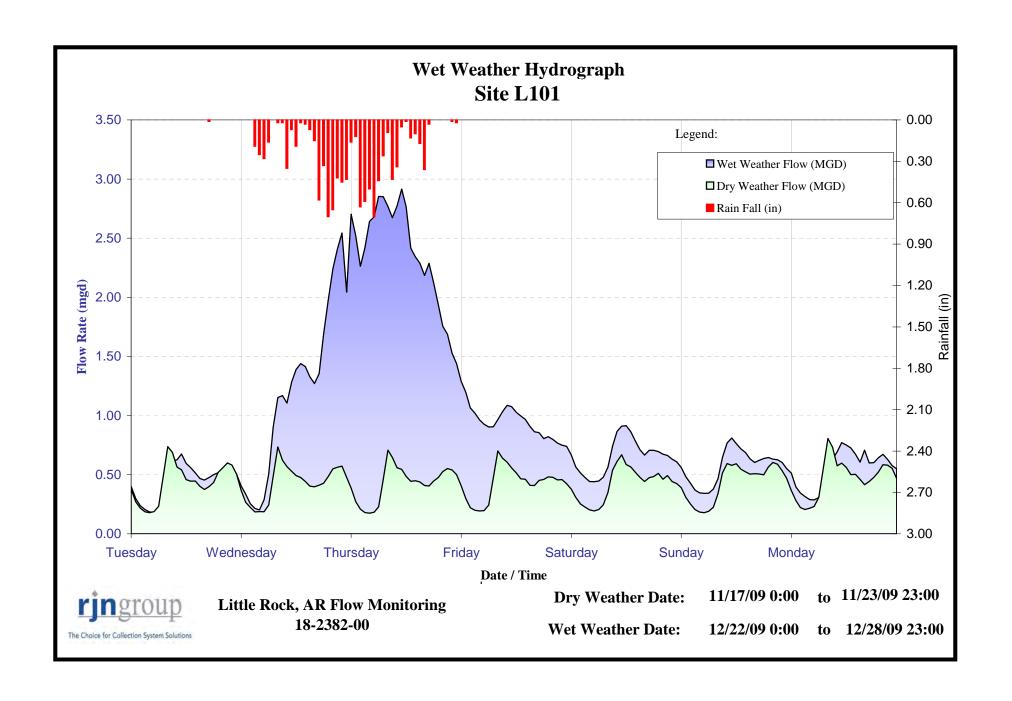


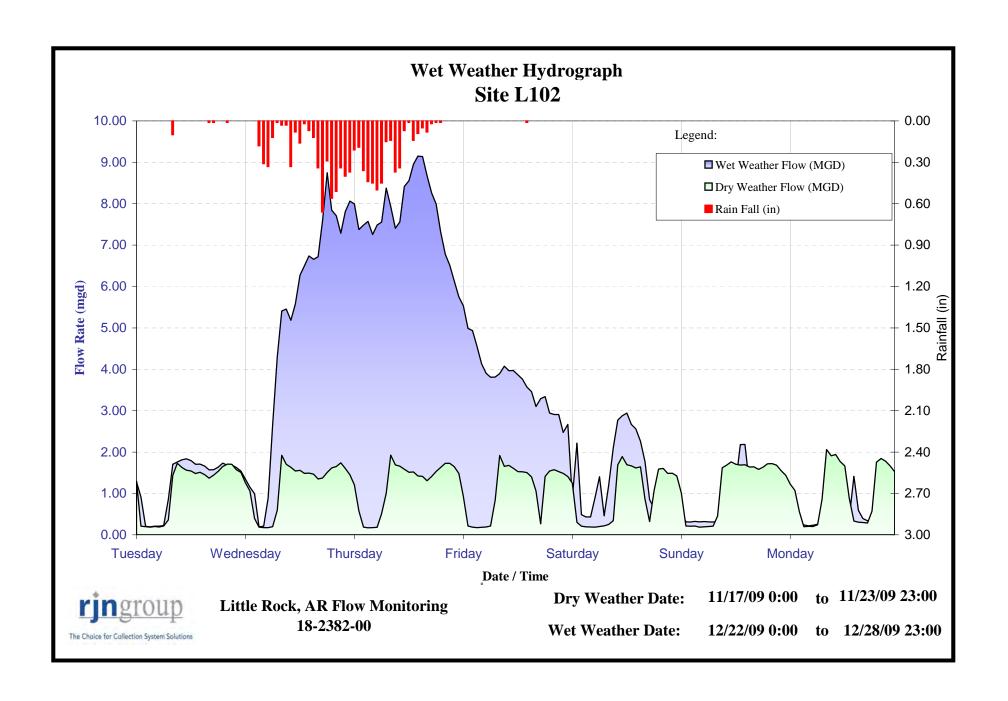


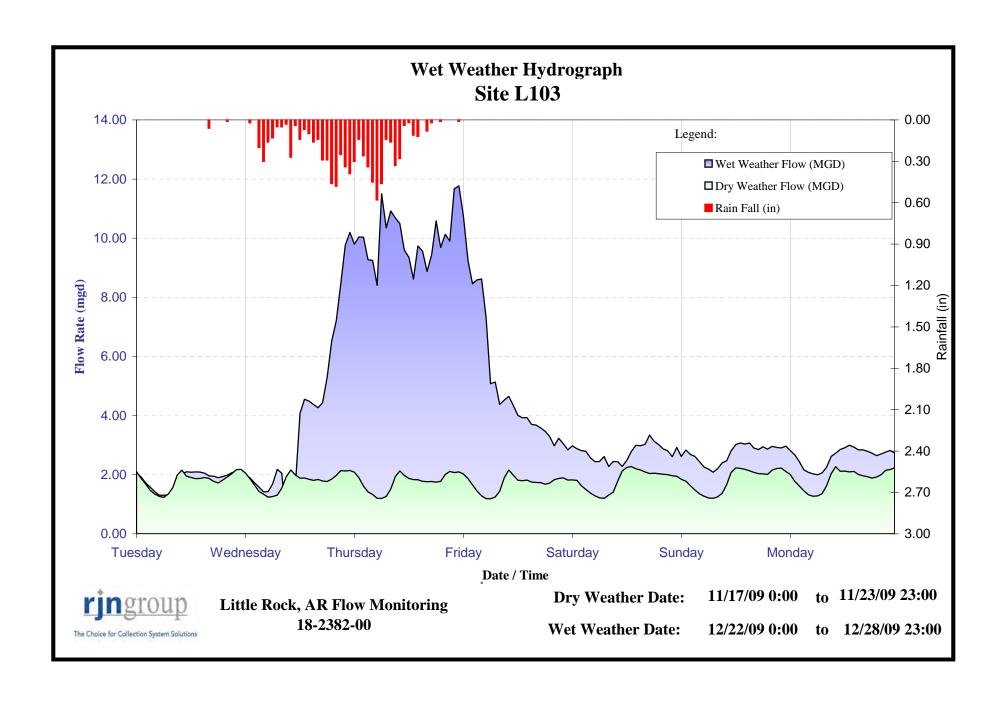


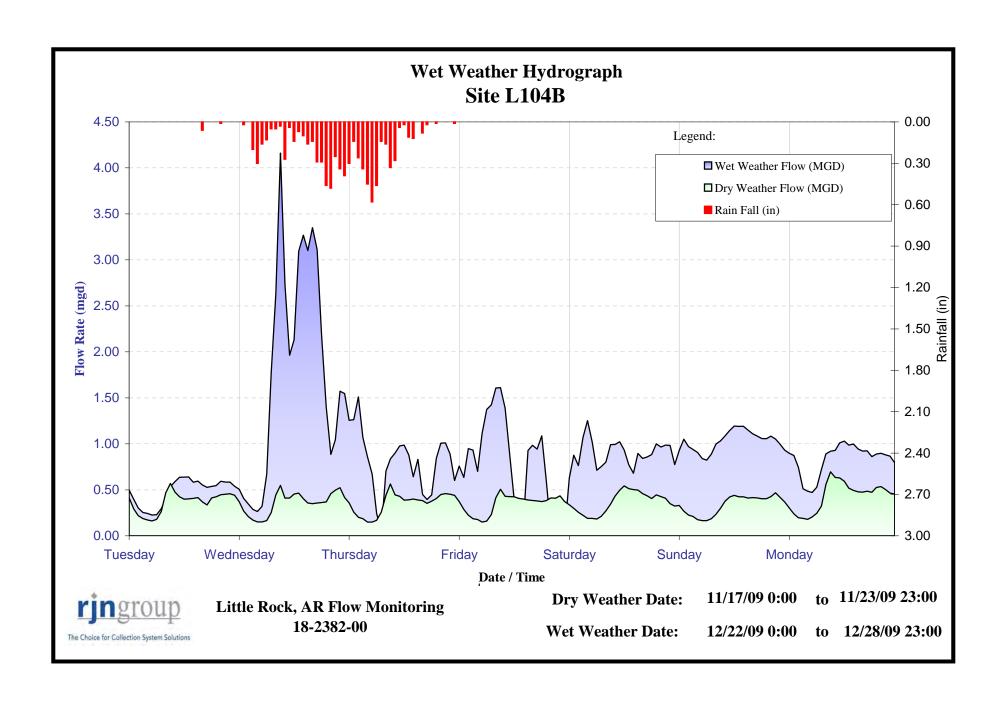


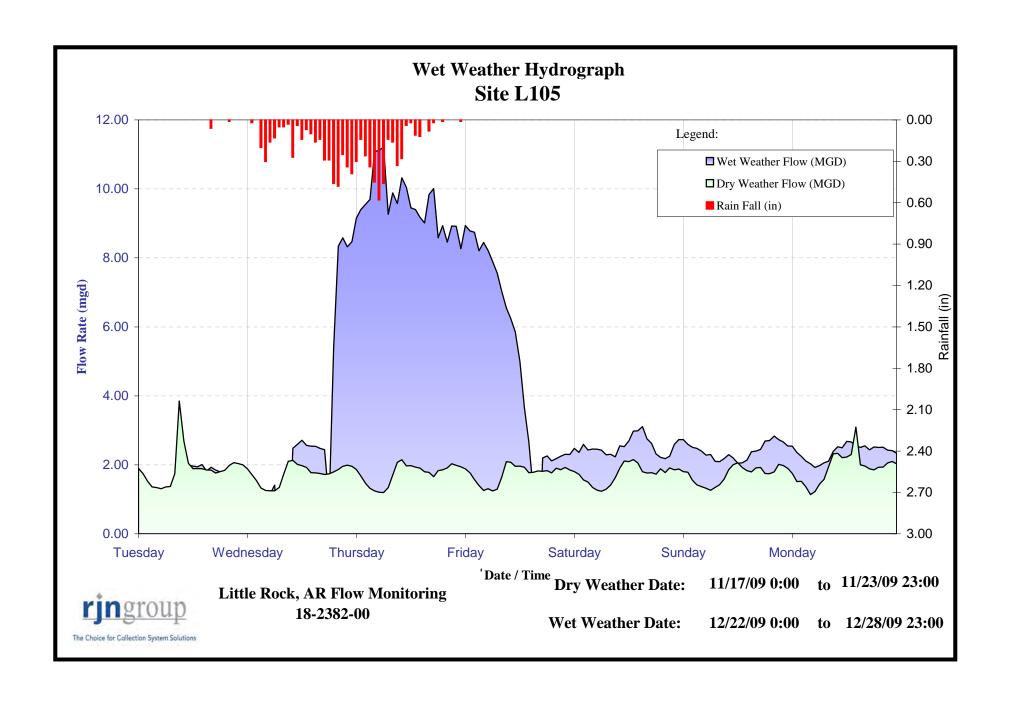


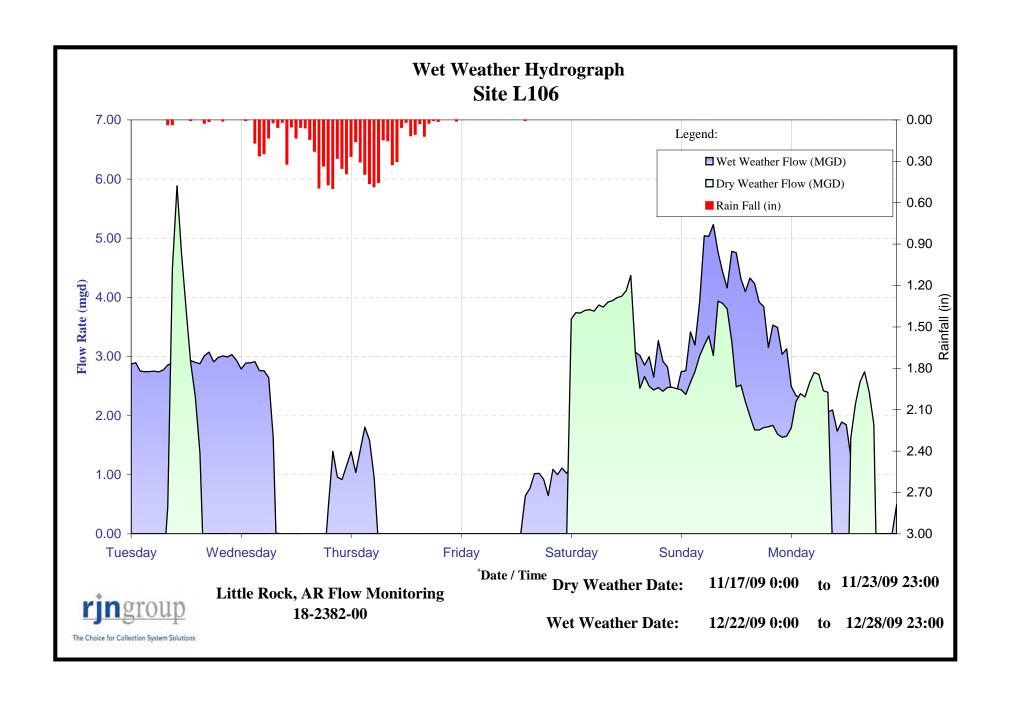


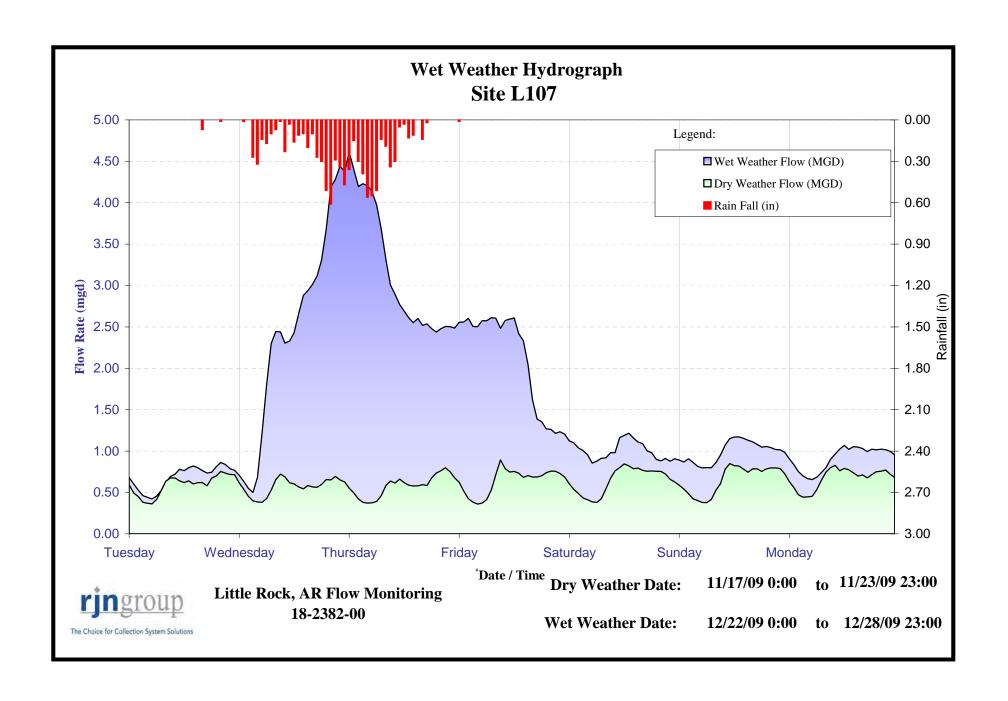


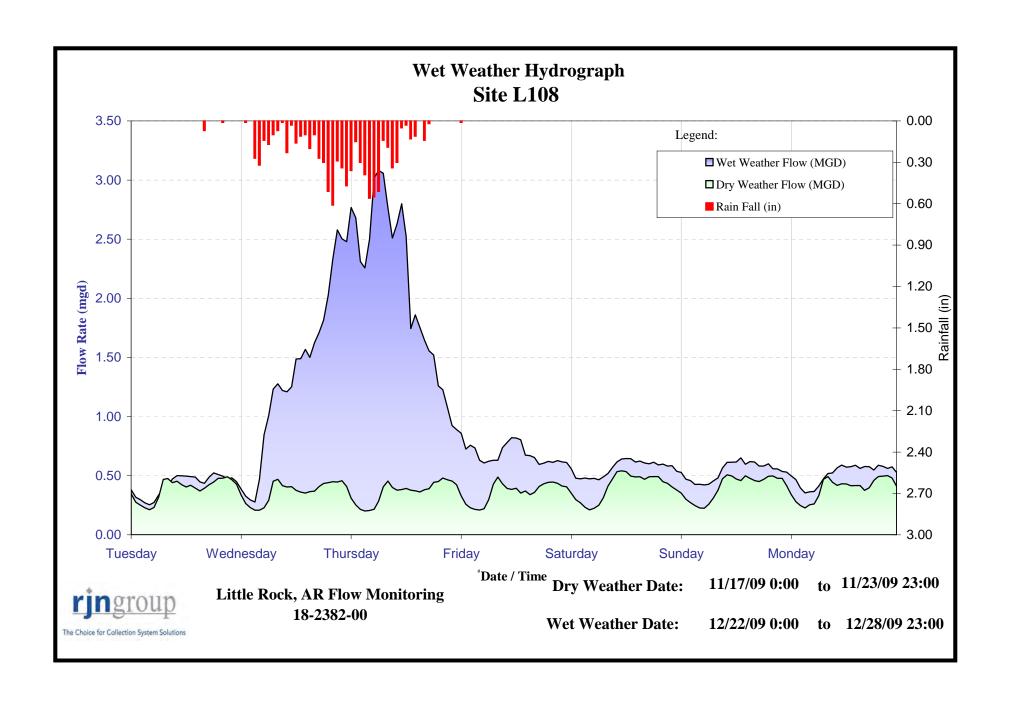


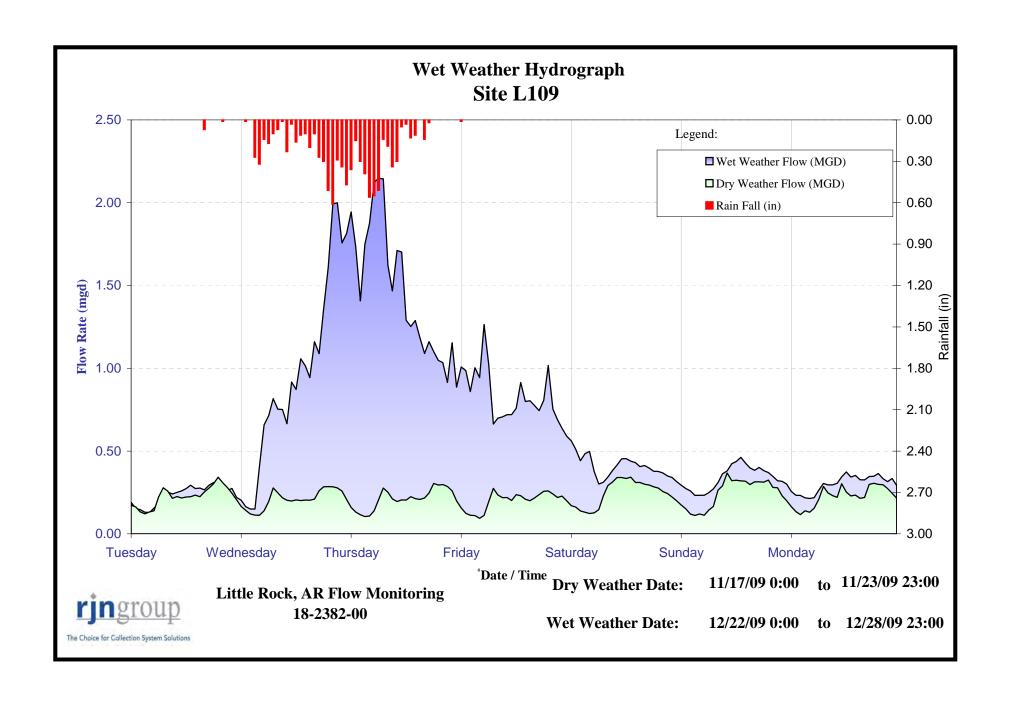


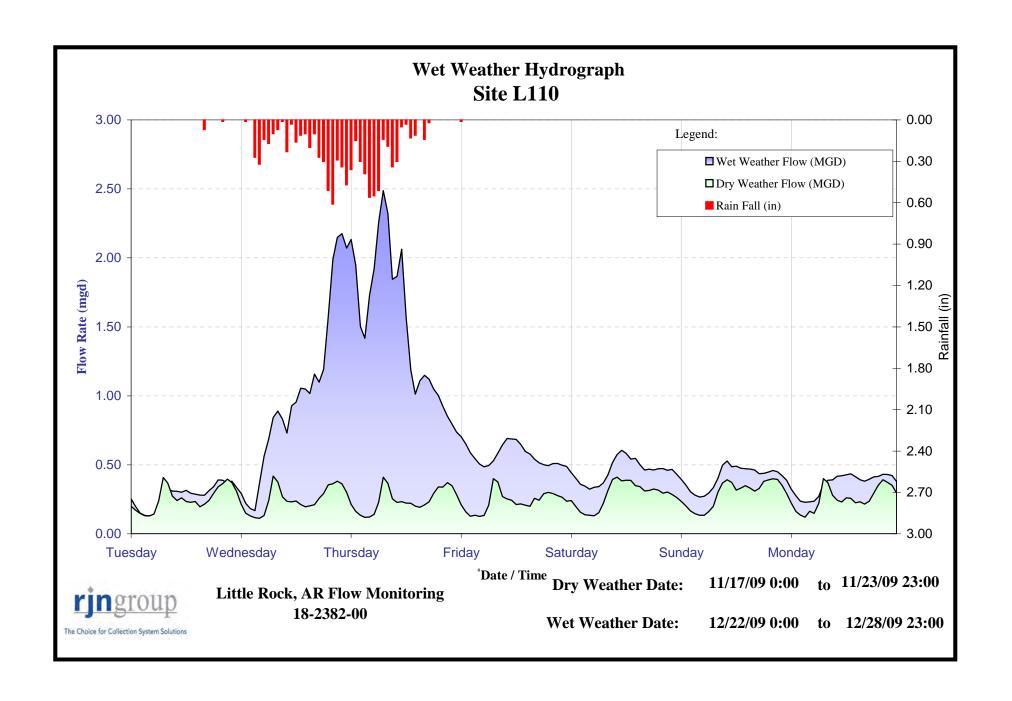


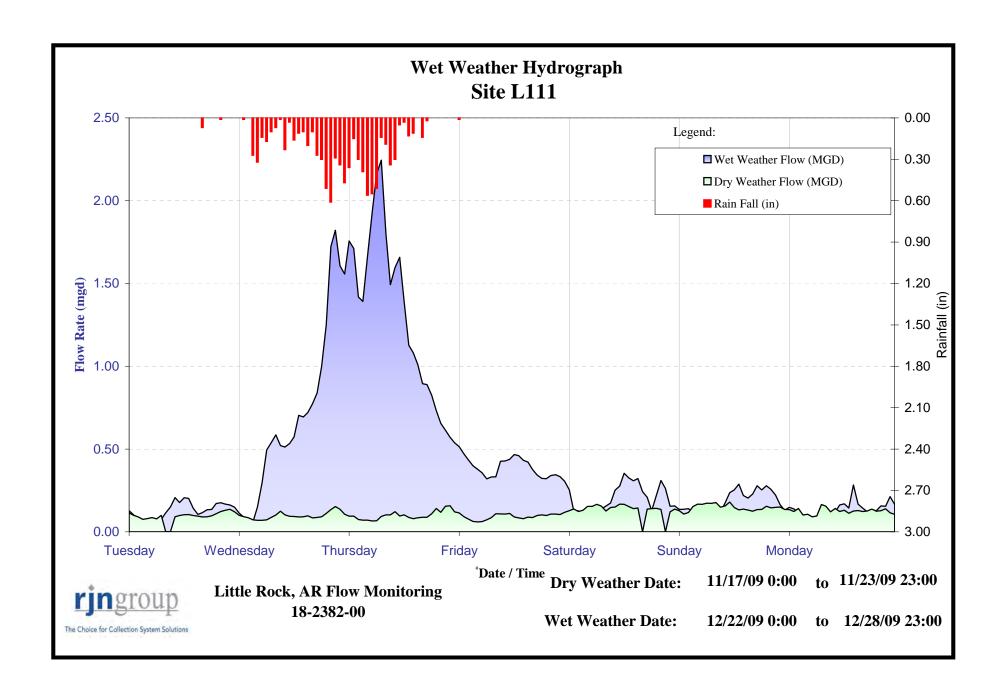


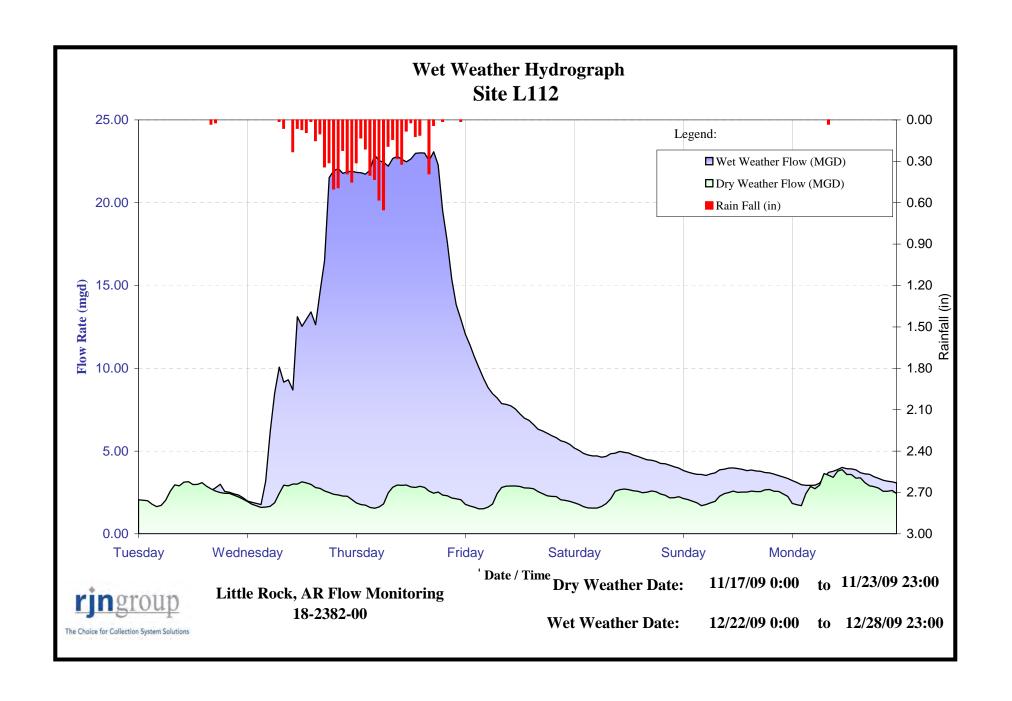


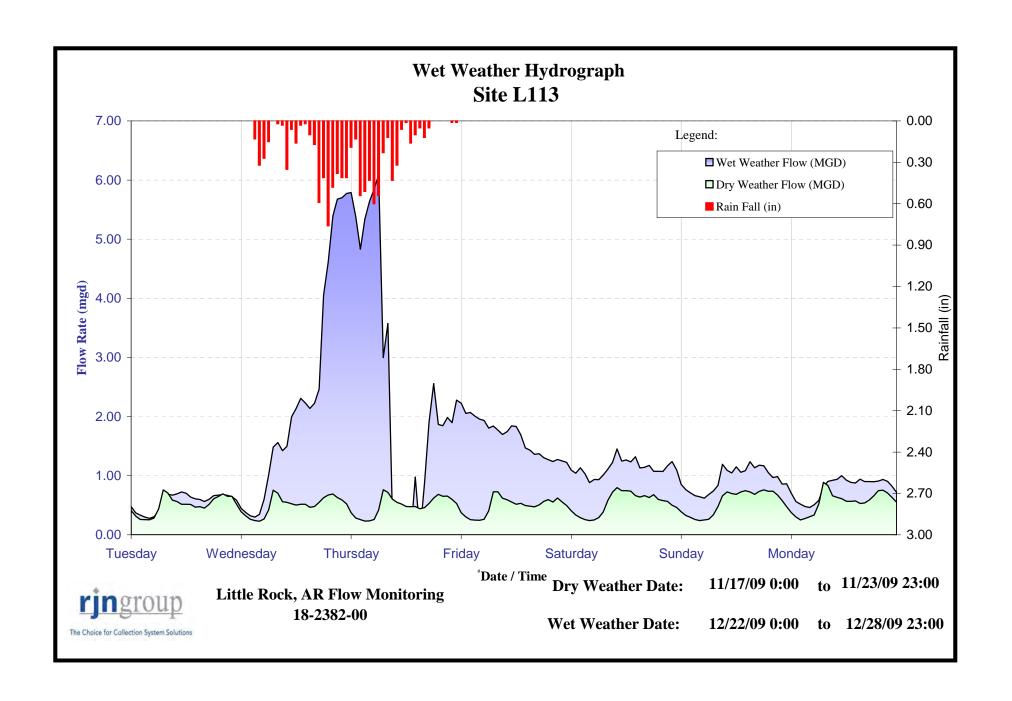


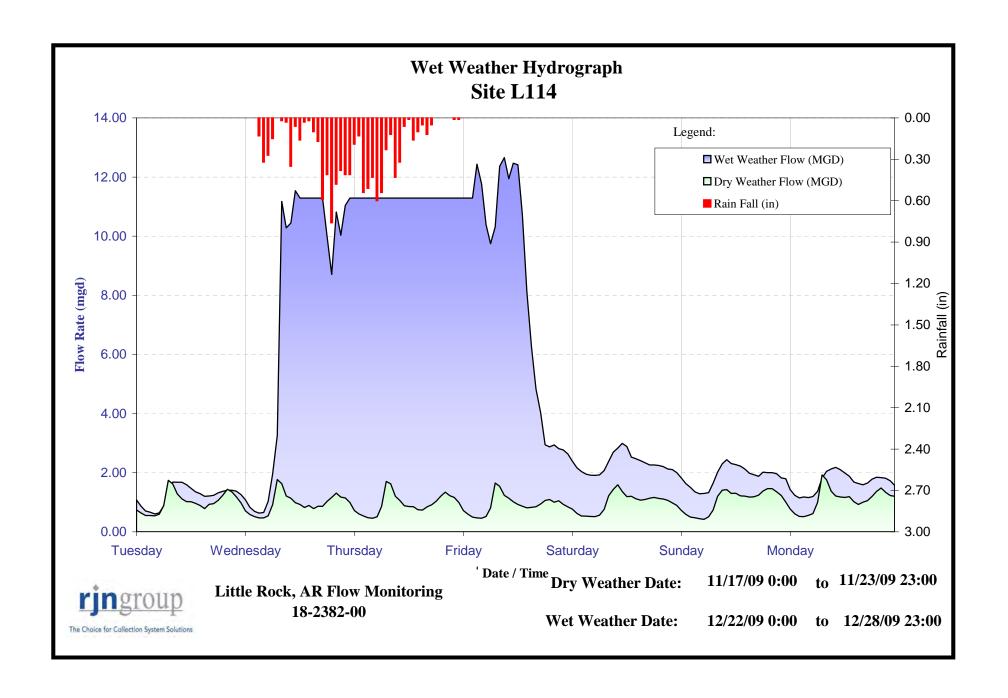


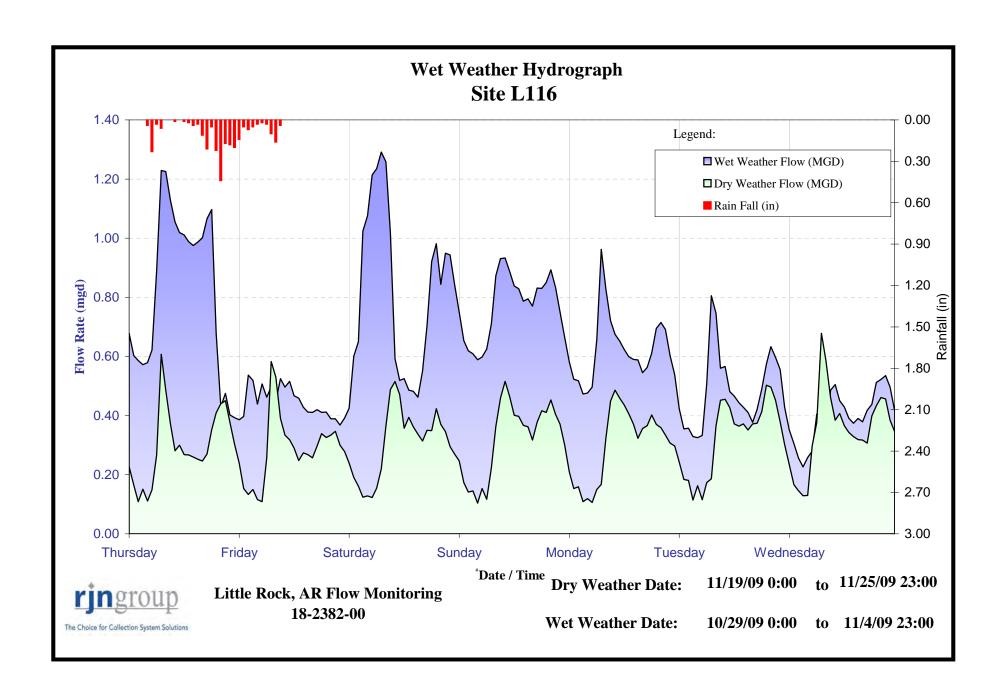


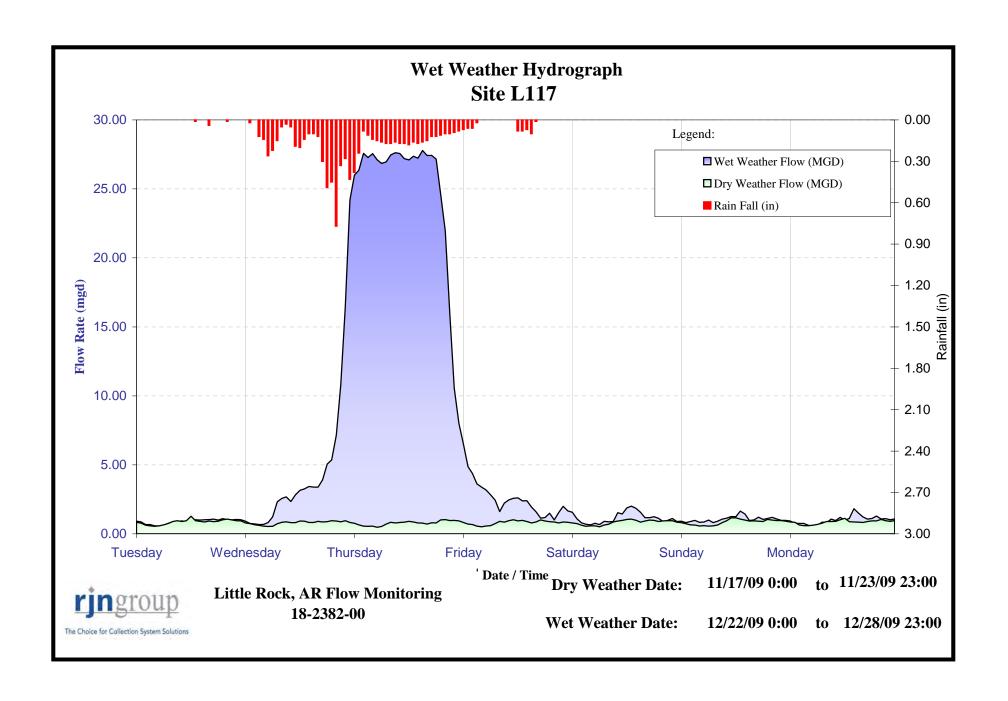


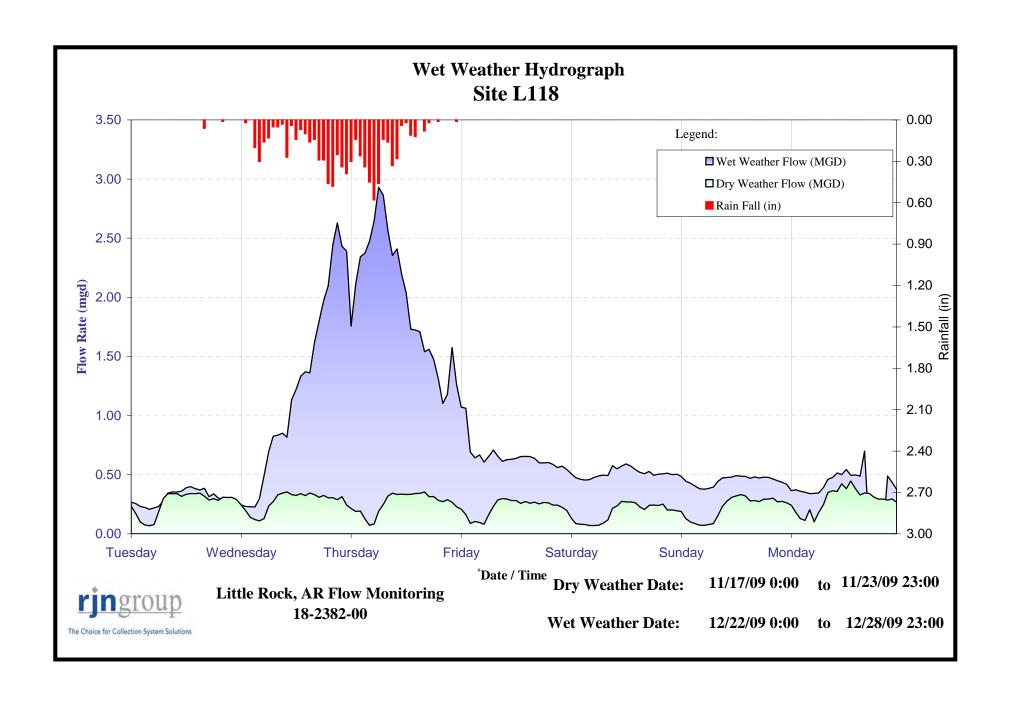


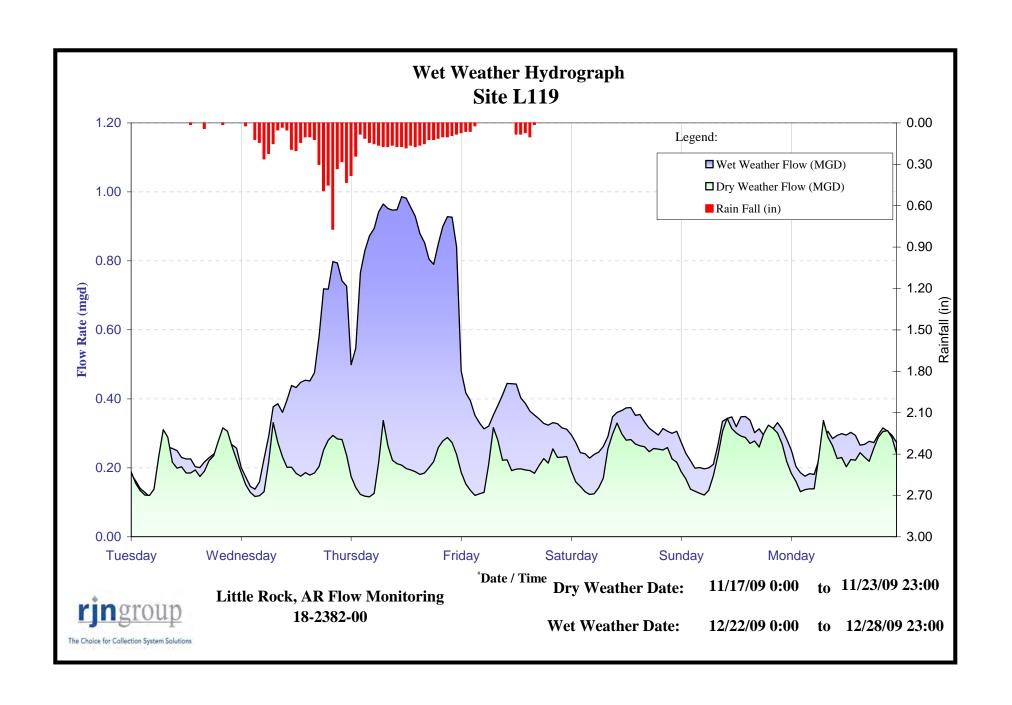


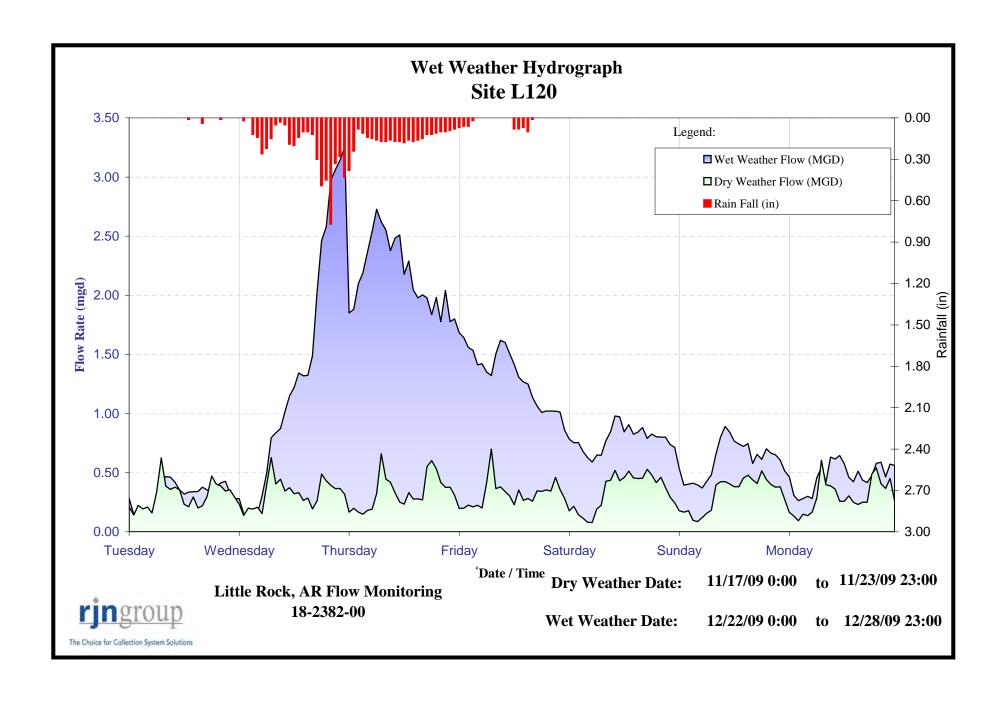


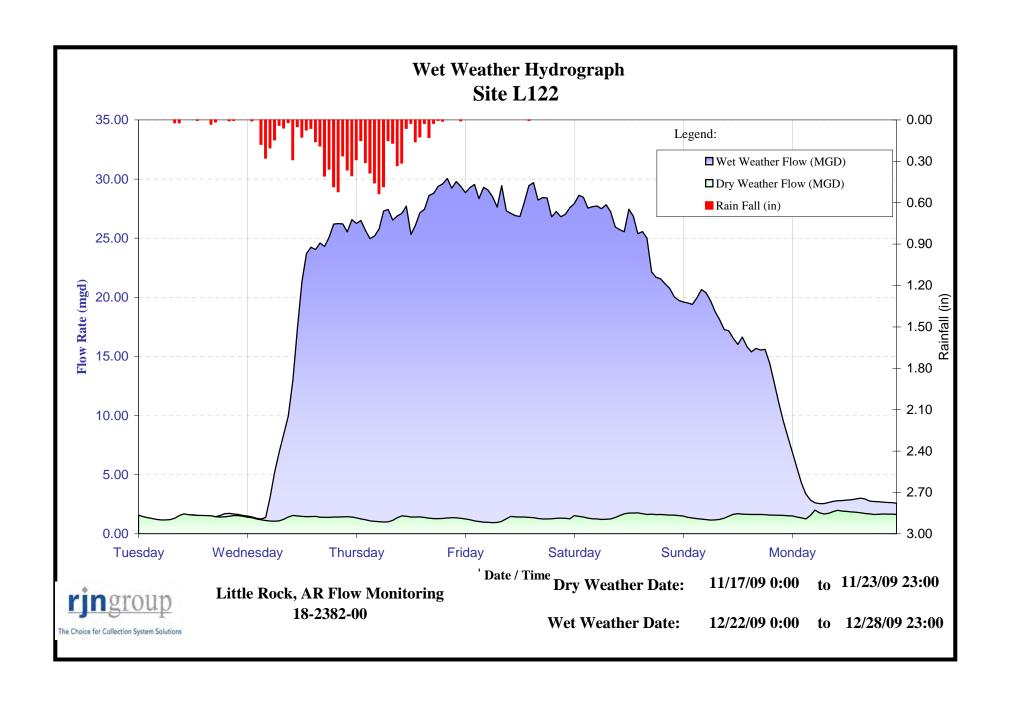


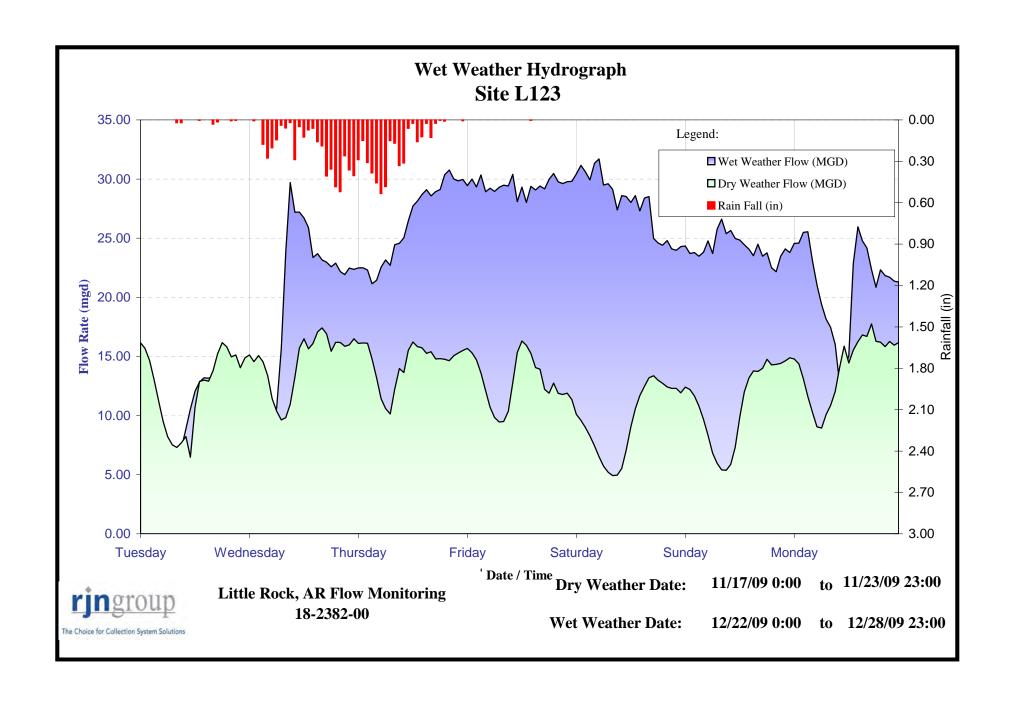


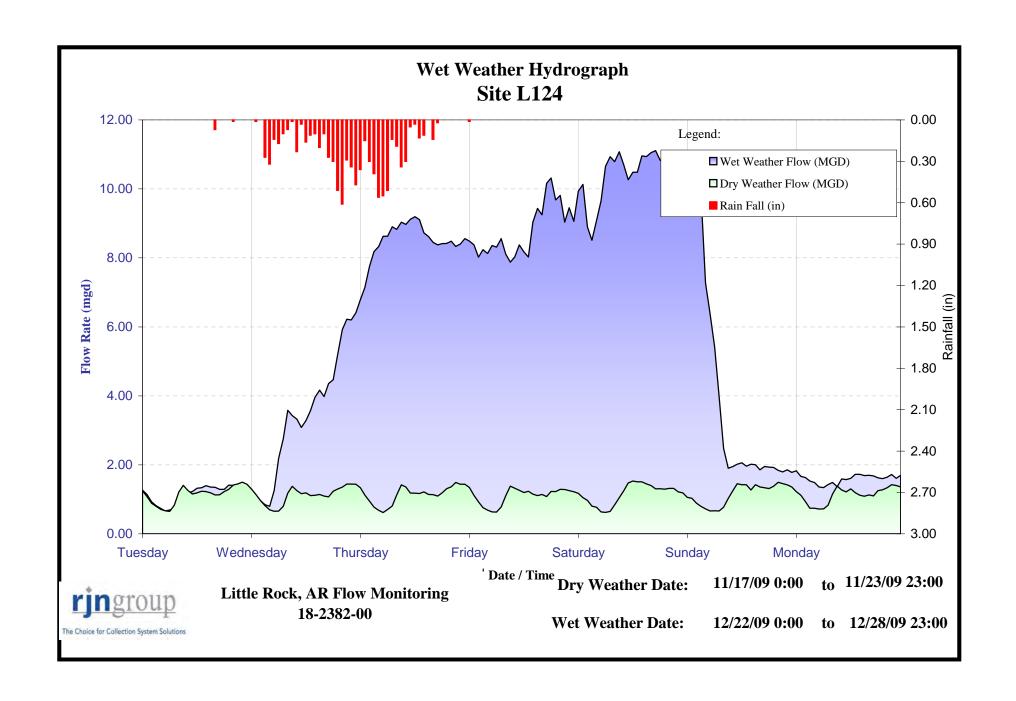


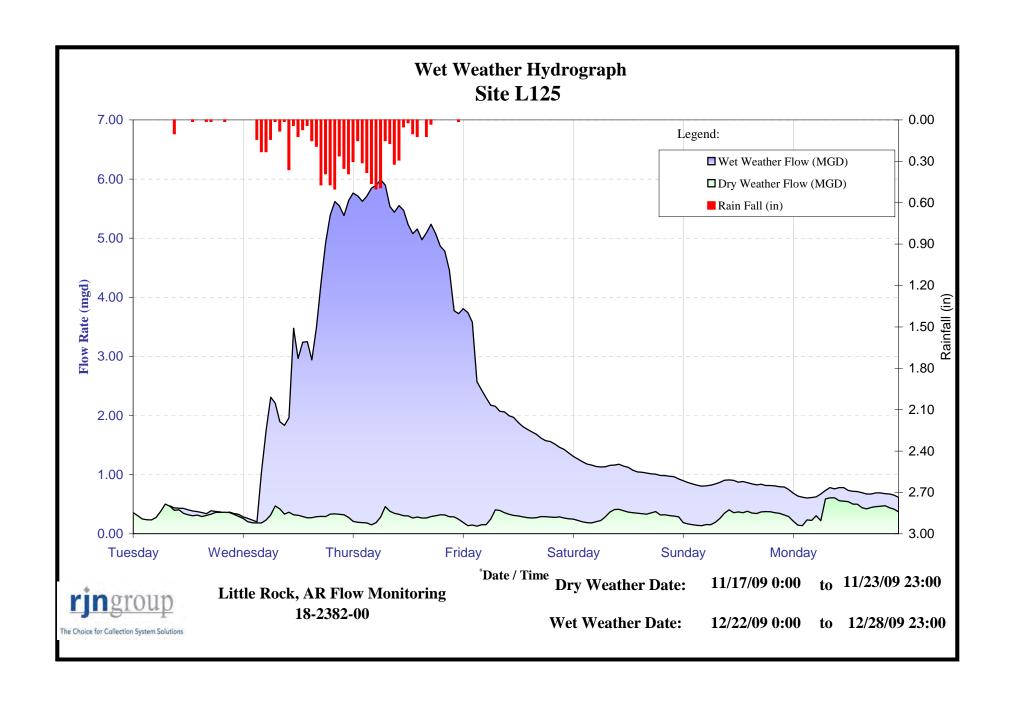


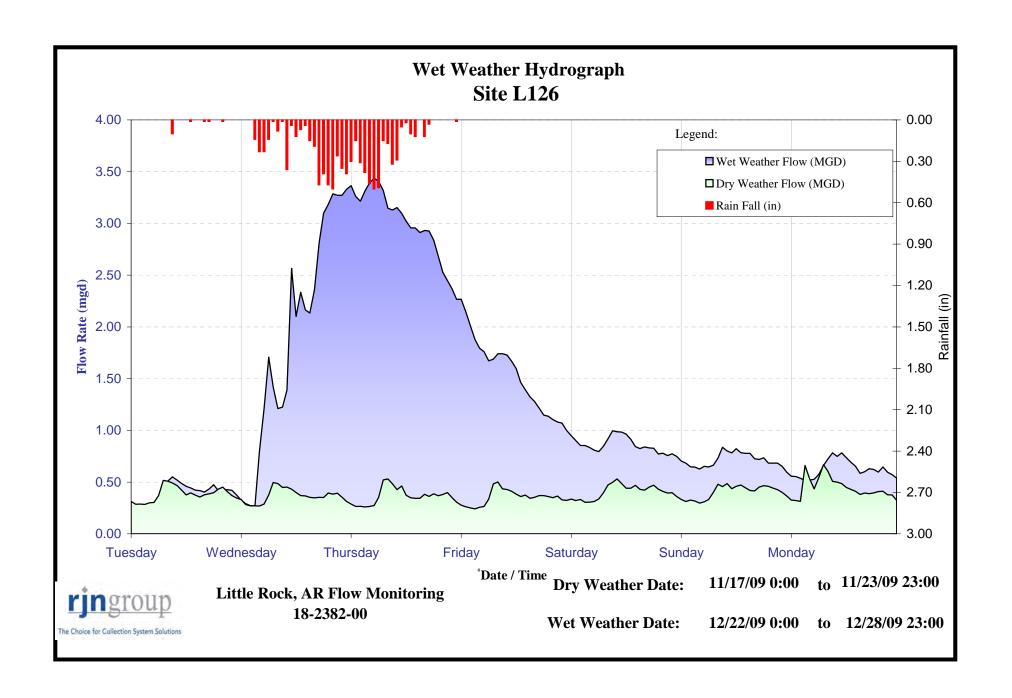












## APPENDIX A FLOW MONITORING SITE REPORTS

